FLOOD INSURANCE STUDY

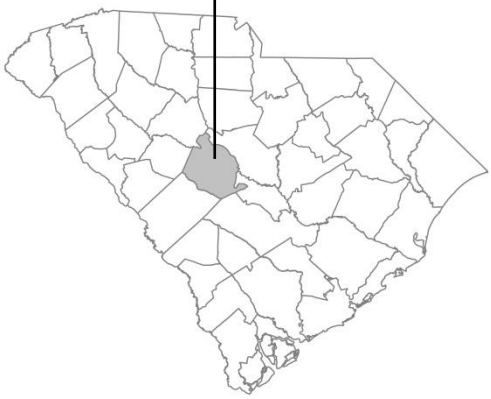
LEXINGTON COUNTY, SOUTH CAROLINA
AND INCORPORATED AREAS

VOLUME 1 OF 2

Community Name	Community Number
BATESBURG-LEESVILLE, TOWN OF	450130
CAYCE, CITY OF	450131
CHAPIN, TOWN OF*	450236
COLUMBIA, CITY OF	450172
GASTON, TOWN OF	450260
GILBERT, TOWN OF	450132
IRMO, TOWN OF	450133
LEXINGTON, TOWN OF	450134
LEXINGTON COUNTY (UNINCORPORATED AREAS)	450129
PELION, TOWN OF	450135
PINE RIDGE, TOWN OF	450136
SOUTH CONGAREE, TOWN OF	450137
SPRINGDALE, TOWN OF	450138
SUMMIT, TOWN OF*	450261
SWANSEA, TOWN OF	450139
WEST COLUMBIA, CITY OF	450140

** NO SPECIAL FLOOD HAZARD AREAS IDENTIFIED*

LEXINGTON COUNTY



REVISED:

PRELIMINARY

OCT 30 2015

Federal Emergency Management Agency

FLOOD INSURANCE STUDY NUMBER
45063CV001D

NOTICE TO
FLOOD INSURANCE STUDY USERS

Communities participating in the National Flood Insurance Program (NFIP) have established repositories of flood hazard data for floodplain management and flood insurance purposes. This Flood Insurance Study (FIS) may not contain all data available within the repository. Please contact the Community Map repository for any additional data.

The Federal Emergency Management Agency (FEMA) may revise and republish part of all of the FIS Report at any time. In addition, FEMA may revise part of the FIS Report by the Letter of Map Revision process, which does not involve republication or redistribution of the FIS. Therefore, users should consult with community officials and check the Community Map repository to obtain the most current FIS Report components.

Initial Countywide FIS
Effective Date: July 17, 1995

Revised Dates: February 9, 2000
February 20, 2002

This preliminary revised Flood Insurance Study contains profiles presented at a reduced scale to minimize reproduction costs. All profiles will be included and printed at full scale in the final published report.

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EXHIBITS – (continued)

FLOOD INSURANCE STUDY
LEXINGTON COUNTY, SOUTH CAROLINA AND INCORPORATED AREAS

1.0 INTRODUCTION

1.1 Purpose of Study

This countywide Flood Insurance Study (FIS) revises and updates the previous countywide FIS/Flood Insurance Rate Map (FIRM) for the geographic area of Lexington County, South Carolina, including the Towns of Batesburg-Leesville, Chapin, Gaston, Gilbert, Irmo, Lexington, Pelion, Pine Ridge, South Congaree, Springdale, Summit, and Swansea; the Cities of Cayce, Columbia, and West Columbia; and the unincorporated areas of Lexington County (hereinafter referred to collectively as Lexington County).

The Towns of Chapin, and Summit have no identified Special Flood Hazard Areas (SFHA).

The Town of Irmo and the Cities of Cayce, and Columbia are located in more than one county. The FIS and FIRM for Lexington County will show the portions of the Town of Irmo and the Cities of Cayce, and Columbia within Lexington County. The remaining portions of these communities lie within Richland County. The flood hazard information for the portions of the Town of Irmo and the Cities of Cayce and Columbia that are located in Richland County is included in the FIS for Richland County, South Carolina, and Incorporated Areas (Reference 1).

The Town of Batesburg-Leesville is located in more than one county. The FIS and FIRM for Lexington County will show the portions of the Town of Batesburg-Leesville within Lexington County. The remaining portion of this community lies within Saluda County. The flood hazard information for the portion of the Town of Batesburg-Leesville that is located in Saluda County is included in the FIS for Saluda County, South Carolina, and Incorporated Areas (Reference 2).

This FIS aids in the administration of the National Flood Insurance Act of 1968 and the Flood Disaster Protection Act of 1973. This FIS has developed flood risk data for various areas of the community that will be used to establish actuarial flood insurance rates. This information will also be used by Lexington County to update existing floodplain regulations as part of the Regular Phase of the National Flood Insurance Program (NFIP), and by local and regional planners to further promote sound land use and floodplain development. Minimum floodplain management requirements for participation in the NFIP are set forth in the Code of Federal regulations at 44 CFR, 60.3.

The Towns of Gaston, and Summit do not participate in the NFIP.

In some states or communities, floodplain management criteria or regulations may exist that are more restrictive or comprehensive than the minimum Federal requirements. In such cases, the more restrictive criteria take precedence, and the State (or other jurisdictional agency) will be able to explain them.

1.2 Authority and Acknowledgments

The sources of authority for this FIS report are the National Flood Insurance Act of 1968 and the Flood Disaster Protection Act of 1973.

The original July 17, 1995, countywide FIS was prepared to include incorporated communities within Lexington County in a countywide FIS. Information on the authority and acknowledgments for each jurisdiction was compiled from their previously printed FIS reports, and is shown below.

Town of Cayce:

The hydrologic and hydraulic analyses for the FIS report dated November 1, 1979, were prepared by the U.S. Army Corps of Engineers (USACE), Charleston District, for the Federal Insurance Administration (FIA), under Inter-Agency Agreement (IAA) No. H-10-77, Project Order No. 5. Field surveys were performed by Heaner Engineering Company, Inc., under supervision of the USACE. That work was completed in June 1978.

The hydrologic and hydraulic analyses for the FIS report dated January 5, 1989, were performed by Mr. Steven M. Bradley, P.E..

City of Columbia:

The hydrologic and hydraulic analyses for the FIS report dated September 2, 1981, were prepared by the USACE, Charleston District, for the FIA, under IAA No. H-10-77, Project Order No. 5. Field surveys were performed by Heaner Engineering Company, Inc., and Triangle Engineering-Architecture Planning, Inc., under supervision of the USACE.

The hydrologic and hydraulic analyses for the FIS report dated February 4, 1987, were performed by the USACE, Charleston District for FEMA.

Town of Irmo:	The hydrologic and hydraulic analyses for the FIS report dated November 1, 1979, were prepared by the USACE, Charleston District, for the FIA, under IAA No H-10-77, Project Order No. 5. Field surveys for that study were performed by Heaner Engineering Company, Inc., under supervision of the USACE. That work was completed in August 1978. The hydrologic and hydraulic analyses for the FIS report dated April 16, 1991, were performed by Mr. Steven M. Bradley, P.E..
Town of Lexington:	The hydrologic and hydraulic analyses for the FIS report date November 1979, were prepared by the USACE, Charleston District, for the FIA, under IAA No H-10-77, Project Order No. 5. Field surveys were performed by Heaner Engineering Company, Inc., under supervision of the USACE. That work was completed in August 1978.
Lexington County (Unincorporated Areas):	The hydrologic and hydraulic analyses for the FIS report dated December 15, 1980, were prepared by the USACE, Charleston District, for the FIA, under IAA No H-10-77, Project Order No. 5. That work was completed in November 1978. The hydrologic and hydraulic analyses for the FIS report dated December 2, 1988, were performed by Mr. Steven M. Bradley, P.E..
Town of Pine Ridge:	The hydrologic and hydraulic analyses for the FIS report dated September 18, 1979, were prepared by the USACE, Charleston District, for the FIA, under IAA No H-7-76, Project Order No. 25, and IAA No H-10-77, Project Order No. 4. . Field surveys were performed by Heaner Engineering Company, Inc., under supervision of the

USACE. That work was completed in March 1978.

The hydrologic and hydraulic analyses for the FIS report dated December 2, 1988, were performed by Mr. Steven M. Bradley, P.E..

Town of South Congaree: The hydrologic and hydraulic analyses for the FIS report date March 28, 1979, were prepared by the USACE, Charleston District, for the FIA, under IAA No H-10-77, Project Order No. 5. That work was completed in March 1978.

The hydrologic and hydraulic analyses for the FIS report dated December 2, 1988, were performed by Mr. Steven M. Bradley, P.E..

Town of Springdale: The hydrologic and hydraulic analyses for the FIS report date November 1979, were prepared by the USACE, Charleston District, for the FIA, under IAA No H-10-77, Project Order No. 5. Field surveys were performed by Heaner Engineering Company, Inc., under supervision of the USACE. That work was completed on May 24, 1978.

City of West Columbia: The hydrologic and hydraulic analyses for the FIS report dated February 15, 1979, were prepared by the USACE, Charleston District, for the FIA, under IAA No H-10-76, Project Order No. 25, and IAA No H-10-77, Project Order No. 4, as amended. That work was completed in December 1977.

For the February 9, 2000, revision, the hydrologic and hydraulic analyses were prepared by Hayes, Seay, Mattern & Mattern, Inc., for FEMA, under Contract No. EMW-95-C-4723. This work was completed in August 1996. Additionally, hydrologic and hydraulic analyses for Tributary to Fourteen Mile Creek were prepared by Carlisle Associates, Inc., and hydraulic analysis of Kinley Creek was prepared by Bradley, Williams and Associates (Reference 3).

For the February 20, 2002, revision, the hydrologic and hydraulic analyses were prepared by Hayes, Seay, Mattern & Mattern, Inc., for FEMA, under Contract No. EMW-95-C-4723, and revised by Dewberry & Davis LLC in response to appeals received. This work was completed in August 2001.

For this revision the hydrologic and hydraulic analyses were prepared by AECOM under contract to South Carolina Department of Natural Resources (SCDNR) for FEMA, under Contract No. EMA-2009-CA-5934. This study was completed in April 2013.

Base map information shown on the FIRM for Lexington County was provided in digital format by Lexington County.

1.3 Coordination

An initial Consultation Coordination Officer's (CCO) meeting is held with representatives from FEMA, the community, and the study contractor to explain the nature and purpose of a FIS, and to identify the streams to be studied by detailed methods. A final CCO meeting is held with representatives from FEMA, the community, and the study contractor to review the results of the study. The final CCO meeting is now referred to as a Preliminary DFIRM Community Coordination (PDCC) meeting.

The dates of the historical initial and final CCO meetings held for Lexington County and the incorporated communities within its boundaries are shown in the following tabulation:

Table 1 – Initial and Final CCO Meeting Dates (Pre-Countywide)

Community Name	Initial CCO Date	Final CCO Date
Cayce, City of	January 1976	April 24, 1979
Columbia, City of	January 1976	August 28, 1980
Irmo, Town of	January 1976	April 25, 1979
Lexington, Town of	January 1976	April 25, 1979
Lexington County (Unincorporated Areas)	January 1976	November 14, 1979
Pine Ridge, Town of	January 1976	February 1, 1979
South Congaree, Town of	January 1976	September 5, 1978

Table 1 – Initial and Final CCO Meeting Dates (Pre-Countywide) – continued

Community Name	Initial CCO Date	Final CCO Date
Springdale, Town of	January 1976	July 1978
West Columbia, City of	January 25, 1977	February 22, 1978

The initial CCO meetings were held with representatives from the FIA, the communities, the USACE, and the Central Midlands Regional Planning Council. The final CCO meetings were held with representatives from the FIA, the communities, and the USACE. Results of the hydraulic analyses were coordinated with the U.S. Geological Survey (USGS).

For the July 17, 1995, FIS, all the communities were notified by letter on March 12, 1993, that a countywide FIS was being initiated.

For the February 9, 2000, revision, an initial CCO meeting was held on September 15, 1994, and was attended by representatives of Hayes, Seay, Mattern & Mattern, Inc., and Lexington County.

In the course of the February 9, 2000, revision, the South Carolina Department of Transportation, the USGS, FEMA, and Dewberry & Davis LLC were contacted to supply relevant information concerning the studied streams. The South Carolina Department of Transportation provided hydraulic analyses as well as many historical high-water marks throughout the county. The USGS provided stream gage data and historical high-water marks. Dewberry & Davis LLC and Lockwood Greene Engineers, Inc., also provided hydrologic and hydraulic analyses for the Congaree River.

For this countywide FIS, an initial CCO (Scoping) meeting was held on January 28, 2010, and attended by representatives of AECOM (the study contractor), FEMA, the Cities of Cayce, Columbia, and West Columbia, Lexington County, and the State NFIP Coordinator. A PDCC meeting was held on --/--/---- to review the results of the study. The meeting was attended by AECOM, FEMA, and Lexington County representatives.

2.0 AREA STUDIED

2.1 Scope of Study

This FIS covers the geographic area of Lexington County, South Carolina.

Historic Study Scopes

All or portions of the following flooding sources have been studied by detailed methods in previous studies: Big Branch, Congaree Creek, Congaree River, First Creek, Fourteen Mile Creek, Kinley Creek, Koon Branch, Lick Fork Branch, Rawls Creek, Red Bank Creek, Saluda River, Savana Branch, Second Creek, Bear Creek, Hunt Branch, Senn Branch, Six Mile Creek, Stoop Creek, Tributary CR-1, Tributary CR-1-1, Tributary K-2, Tributary R-2, Tributary SM-2, Tributary SM-3, Tributary SM-5, Tributary to Fourteen Mile Creek, Twelve Mile Creek, Yost Creek, and Lake Murray.

For the July 17, 1995 FIS, updated analyses were included for the following flooding sources:

Table 2 – Flooding Sources Studied by Detailed Methods (July 17, 1995)

Flooding Source	Downstream Limit	Upstream Limit	Length (miles)
Fourteen Mile Creek	Confluence with Twelve Mile Creek	0.6 miles upstream of Old Chapin Road	6.8
Savana Branch	Confluence with Congaree Creek	At Edmund Highway	1.1
Senn Branch	Approximately 560 feet downstream of Ephrata Drive	Approximately 20 feet upstream of Hebron Drive	1.3
Stoop Creek	Confluence with Saluda River	Approximately 0.5 miles upstream of Interstate-26	1.8
Tributary K-2	Approximately 20 feet downstream of Piney Grove Road	Approximately 0.5 miles upstream of Piney Grove Road	0.5
Tributary SM-2	Confluence with Six Mile Creek	Approximately 130 feet upstream of Old Frink Street	0.6
Tributary SM-3	Approximately 290 feet downstream of Route-302	Approximately 0.4 miles upstream of Lexington Drive Dam	1.2
Twelve Mile Creek	Confluence with Saluda River	Approximately 1.0 miles upstream of Mineral Springs Road	5.6

Table 2 – Flooding Sources Studied by Detailed Methods (July 17, 1995) – continued

Flooding Source	Downstream Limit	Upstream Limit	Length (miles)
Yost Creek	Confluence with Rawls Creek	Approximately 60 feet upstream of Lincreek Drive	2.1

In addition, a Zone AO (Depth 2.0 feet) designation was added near the confluence of Savana Branch with Congaree Creek.

The July 17, 1995 FIS incorporated the effects of annexations or deannexations of land by Lexington County; the Cities of Cayce, Columbia and West Columbia; and the Towns of Batesburg, Leesville, Chapin, Gaston, Irmo, Lexington, Pine Ridge, Pelion, South Congaree, and Swansea. The January 17, 1995 FIS also incorporated the determinations Letters of Map Revision (LOMRs) issued by FEMA for the following projects:

Table 3 – Incorporated Letters of Map Revision (July 17, 1995)

Case Number	Flooding Sources	Community Name	Effective Date
*	Six Mile Creek	Town of Springdale	October 19, 1987
*	Kinley Creek	Lexington County	July 13, 1989
*	Red Bank Creek	Lexington County	April 11, 1990
*	Tributary CR-1	City of Cayce	November 9, 1990
92-04-008P	Kinley Creek	Lexington County	December 20, 1991
94-04-211P	Rawls Creek	Lexington County	December 7, 1994

*** – denotes data unavailable**

For the February 9, 2000, FIS, updated analyses were included for the following flooding sources:

Table 4 – Flooding Sources Studied by Detailed Methods (February 9, 2000)

Flooding Source	Downstream Limit	Upstream Limit	Length (miles)
Bear Creek	Confluence with Second Creek	Confluence of Hunt Branch	1.8
Congaree Creek	Confluence with Congaree River	Immediately upstream of Platt Springs Road	18.1
First Creek	Approximately 800 feet downstream of Dogwood Road	Approximately 250 feet upstream of Goodwin Dam	7.3
Fourteen Mile Creek	Approximately 0.6 miles upstream of Old Chapin Road	Immediately upstream of Wise Ferry Road	2.3
Hunt Branch	Confluence with Bear Creek	Approximately 350 feet upstream of Darden Pond Dam	0.9
Kinley Creek	At Piney Grove Road	At Beaver Dam Road	1.2
Lick Fork Branch	Confluence with Red Bank Creek (Durham Pond)	Immediately downstream of Kitti Wake Drive Pond	2.6
Red Bank Creek	Confluence with Congaree Creek	Immediately downstream of Calk's Ferry Road	12.0
Saluda River	Confluence with Congaree River	Immediately upstream of Interstate-20	4.7
Saluda River (Lake Murray)	Entire shoreline within Lexington County	Entire shoreline within Lexington County	11.5
Savana Branch	Confluence with Congaree Creek	Approximately 75 feet upstream of St. Davids Church Road	5.1
Second Creek	Confluence with First Creek	Confluence of Bear Creek	3.4
Six Mile Creek	Confluence with Congaree Creek	Confluence of Tributary SM-2	2.2

Table 4 – Flooding Sources Studied by Detailed Methods (February 9, 2000) – continued

Flooding Source	Downstream Limit	Upstream Limit	Length (miles)
Tributary to Fourteen Mile Creek	Confluence with Fourteen Mile Creek	Approximately 1,890 feet upstream of confluence with Fourteen Mile Creek	0.4
Twelve Mile Creek	At Gibson Pond	Approximately 2,500 feet upstream of Taylor Mill Pond Dam	8.7

Floodplain boundaries of streams that were previously studied by detailed methods and were not restudied in the February 9, 2000, revision, were redelineated using more up-to-date topographic information. (Reference 4).

The February 9, 2000, revision, also incorporated LOMRs issued by FEMA for the following projects:

Table 5 – Incorporated Letters of Map Revision (February 9, 2000)

Case Number	Flooding Sources	Community Name	Effective Date
98-04-143P	Long Branch Long Branch Tributary A	Town of Lexington	July 8, 1998

For the February 20, 2002, FIS, an updated analysis by detailed methods was included for the Congaree River for its entire reach within the community. Numerous flooding sources were studied by approximate methods.

Current Study Scopes

For this revision streams were studied by approximate, limited detailed and detailed methods, these are listed in Table 6, Table 7, and Table 8 , respectively.

Table 6 – Flooding Sources Studied by Approximate Methods (Current Revision)

Flooding Source	Length (miles)
Beach Creek	0.6
Bear Creek 2	3.0
Big Beaver Creek	3.7
Big Beaver Creek Tributary 1	2.0
Big Beaver Creek Tributary 1-1	0.4
Big Beaver Creek Tributary 2	0.6
Big Branch	0.7
Black Creek	21.0
Black Creek Tributary 6	0.4
Boggy Branch	2.5
Boggy Branch 2	0.8
Bull Swamp Creek	11.0
Bull Swamp Creek Tributary 3	1.3
Carney Branch	1.7
Carneys Creek	1.1
Cedar Creek	9.3
Cedar Creek Tributary 4	1.5
Chinquapin Creek	0.7
Clemons Creek	0.4
Congaree Creek	2.1
Congaree Creek Tributary 1	1.4
Congaree Creek Tributary 1-1	0.4
Congaree Creek Tributary 2	0.7
Congaree Creek Tributary 3	1.0
Congaree Creek Tributary 3-1	0.3
Congaree Creek Tributary 4	0.4
Congaree Creek Tributary 5	0.7

Table 6 – Flooding Sources Studied by Approximate Methods (Current Revision) – continued

Flooding Source	Length (miles)
Congaree Creek Tributary 6	0.5
Congaree Creek Tributary 7	0.8
Crawford Branch	0.8
Double Branch	1.3
Duncan Creek	3.9
Duncan Creek Tributary 1	0.5
Duncan Creek Tributary 2	0.4
Duncan Creek Tributary 4	0.7
East Fork Scouter Branch	3.6
East Fork Scouter Branch Tributary 1	0.4
East Fork Scouter Branch Tributary 2	1.0
Eleven Mile Creek	0.5
First Creek	1.8
First Creek Tributary 1	0.4
First Creek Tributary 2	0.8
First Creek Tributary 2-1	0.2
First Creek Tributary 3	0.4
First Creek Tributary 4	0.3
Fourteen Mile Creek Tributary 1	0.4
Fourteen Mile Creek Tributary 2	0.4
Fourth Creek	1.3
Gin Branch	1.0
Hellhole Creek	3.8
Hollow Creek	5.8
Hollow Creek Tributary 1	0.6
Hollow Creek Tributary 2	0.8
Hood Branch	2.6

Table 6 – Flooding Sources Studied by Approximate Methods (Current Revision) – continued

Flooding Source	Length (miles)
Hood Branch Tributary 1	1.3
Horse Creek	5.8
Horsepen Creek	2.3
Horsepen Creek Tributary 1	0.3
Hunt Branch	0.9
Hunter Branch	3.4
Jackson Branch	3.6
Kinley Creek Tributary 3	0.4
Lake Murray Spillway	0.9
Lick Creek	2.4
Lick Fork Branch	0.7
Lightwood Knot Creek	10.7
Lightwood Knot Creek Tributary 2	0.6
Little Black Creek	5.4
Little Creek	5.3
Little Horse Creek	3.6
Long Creek	1.4
Long Creek Tributary 1	0.4
Long Creek Tributary 2	0.4
Lynch Branch	2.2
Mare Branch	1.4
Mill Creek	0.8
Pond Branch	1.4
Pond Branch 2	7.6
Red Bank Creek	1.0
Red Bank Creek Tributary 2	1.2
Red Bank Creek Tributary 3	0.7

Table 6 – Flooding Sources Studied by Approximate Methods (Current Revision) – continued

Flooding Source	Length (miles)
Reedy Branch	0.9
Risters Creek	3.5
Risters Creek Tributary 1	0.4
Risters Creek Tributary 2	0.3
Risters Creek Tributary 3	0.4
Risters Creek Tributary 4	0.3
Risters Creek Tributary 5	0.6
Risters Creek Tributary 6	0.5
Risters Creek Tributary 7	0.4
Rock Creek	2.9
Rock Creek Tributary	0.3
Rocky Branch	1.5
Saluda River	1.3
Sandy Run	3.2
Sandy Run Tributary 3	2.3
Savana Branch Tributary 1	0.4
Savana Branch Tributary 2	0.4
Savany Hunt Creek	2.3
Savany Hunt Creek Tributary 1	0.9
Savany Hunt Creek Tributary 2	0.5
Six Mile Creek	0.8
Six Mile Creek Tributary 3	1.2
Six Mile Creek Tributary 4	1.0
Six Mile Creek Tributary 6	1.4
Six Mile Creek Tributary 7	0.5
Six Mile Creek Tributary 8	1.1
Six Mile Creek Tributary 8-1	0.3

Table 6 – Flooding Sources Studied by Approximate Methods (Current Revision) – continued

Flooding Source	Length (miles)
Smith Branch	1.0
Stevens Creek (modeled Millers Branch)	0.8
Third Creek	4.5
Thrasher Branch	0.7
Twelve Mile Creek	1.5
Twelve Mile Creek Tributary 1	0.4
Twelve Mile Creek Tributary 2	0.9
Twelve Mile Creek Tributary 3	1.7
Twelve Mile Creek Tributary 3-1	1.5
Twelve Mile Creek Tributary 4	1.5
Twenty Mile Creek	0.3
Wateree Creek	0.5
Wateree Creek Tributary 7	1.8
Wateree Creek Tributary 10	0.3
West Creek Tributary 13	0.7
Whetstone Creek	3.2
Wolf Pit Branch	2.2

Table 7 – Flooding Sources Studied by Limited Detailed Methods (Current Revision)

Flooding Source	Downstream Limit	Upstream Limit	Length (miles)
Chinquapin Creek [1]	Confluence with North Fork Edisto River	Approximately 0.7 miles downstream of the Lexington–Aiken–Saluda County Boundary	10.0

Table 7 – Flooding Sources Studied by Limited Detailed Methods (Current Revision) – continued

Flooding Source	Downstream Limit	Upstream Limit	Length (miles)
Double Branch	Approximately 300 feet downstream of Greenwood Road	Approximately 450 feet upstream of Dew Avenue	0.8
Double Branch Tributary 1	Confluence with Double Branch	Approximately 120 feet upstream of Hook Avenue	0.4
Dry Creek	Approximately 0.5 miles downstream of Interstate-26	Immediately downstream of Gardners Terrace Road	2.6
Dry Creek Tributary 3	Confluence with Dry Creek	Approximately 620 feet downstream of Gardners Terrace Road	0.5
Kinley Creek	At Beaver Dam Road	Approximately 0.3 miles downstream of Archers Lane	0.9
Long Branch	Approximately 0.5 miles upstream of confluence with Fourteen Mile Creek	Approximately 0.2 miles upstream of Wise Ferry Road	1.1
North Fork Edisto River [1]	At the Lexington–Aiken–Orangeburg County Boundary	Confluence of Chinquapin Creek	31.8
Risters Creek [2]	At the Lexington–Richland County Boundary	Approximately 250 feet upstream of Dan Comalander Road	1.0
Risters Creek Tributary [2]	Confluence with Risters Creek	Approximately 1,300 feet upstream of Haltiwanger Road	0.5
Rocky Creek	Approximately 0.3 miles downstream of Highway 378	Approximately 0.6 miles upstream of Calks Ferry Road	1.5
Saluda River Tributary 1	Confluence with Saluda River	Approximately 200 feet downstream of Corley Mill Road	1.5

Table 7 – Flooding Sources Studied by Limited Detailed Methods (Current Revision) – continued

Flooding Source	Downstream Limit	Upstream Limit	Length (miles)
Saluda River Tributary 1-1	Confluence with Saluda River Tributary 1	Approximately 400 feet upstream of Bent Ridge Court	0.6
Saluda River Tributary 1-2	Confluence with Saluda River Tributary 1	Approximately 0.2 miles upstream of Meadowbrook Lane	0.8
Saluda River Tributary 1-2-2	Confluence with Saluda River Tributary 1-2	Approximately 0.2 miles upstream of confluence with Saluda River Tributary 1-2	0.2
Toms Branch	Approximately 0.5 miles downstream of Railroad	Approximately 200 feet downstream of Highway 321	4.1
Toms Branch Tributary 1	Confluence with Toms Branch	Immediately downstream of Colony Park Lane	0.9
Turkey Creek	Confluence with Red Bank Creek	Approximately 0.3 miles upstream of Longs Pond Road	1.7
Wateree Creek [2]	At the Lexington–Richland County Boundary	At the Lexington–Richland County Boundary	0.3
Wateree Creek Tributary 7 [2]	At the Lexington–Richland County Boundary	Approximately 950 feet upstream of the Lexington–Richland County Boundary	0.2

[1] – denotes flooding information incorporated from Aiken County, SC FIS

[2] – denotes flooding information incorporated from Richland County, SC FIS

Table 8 – Flooding Sources Studied by Detailed Methods (Current Revision)

Flooding Source	Downstream Limit	Upstream Limit	Length (miles)
Broad River [1]	Confluence with Congaree River / Saluda River	Approximately 1,800 feet upstream of confluence with Congaree River / Saluda River	0.4
Congaree River [1]	At the Lexington–Richland County Boundary	Confluence of Broad River / Saluda River	11.2
Kinley Creek	Confluence with Saluda River	Approximately 1,700 feet upstream of Piney Grove Road	2.4
Kinley Creek Tributary 1	Confluence with Kinley Creek	Approximately 360 feet downstream of Interstate-26	1.2
Kinley Creek Tributary 2	Confluence with Kinley Creek	Immediately downstream of Interstate-26	1.4
Rawls Creek [1]	Approximately 430 feet upstream of Cressfell Road	At the Lexington–Richland County Boundary	0.1
Rawls Creek Tributary 2 [1]	Approximately 1,365 feet upstream of confluence with Rawls Creek	At the Lexington–Richland County Boundary	0.1
Saluda River [1]	Confluence with Broad River / Congaree River	Approximately 1,000 feet upstream of Interstate-26	3.5
Stoop Creek [1]	Immediately upstream of Fernandina Road	At the Lexington–Richland County Boundary	0.1

[1] – denotes flooding information incorporated from Richland County, SC FIS

This revision also incorporates LOMRs issued by FEMA for the following projects:

Table 9 – Incorporated Letters of Map Revision (Current Revision)

Case Number	Flooding Sources	Community Name	Effective Date
00-04-101P	Twelve Mile Creek	Lexington County	12/15/2000
01-04-501P	Fourteen Mile Creek	Lexington County	07/26/2002
03-04-515P	Senn Branch	Lexington County	04/15/2004
04-04-139P	Tributary CR-1	City of Cayce; Lexington County	10/06/2004
06-04-BI42P	Beach Creek Unnamed Tributary to Beach Creek	Lexington County	01/31/2007
06-04-BM33P	Tributary SM-3	Lexington County	01/11/2007
06-04-BQ42P	Unnamed Tributary of Twelve Mile Creek	Lexington County	01/25/2007
07-04-0471P	Fourteen Mile Creek Unnamed Tributary to Fourteen Mile Creek	Lexington County	01/25/2007
07-04-5295P	Tributary SM-6	Lexington County; Town of Springdale	10/31/2007
07-04-5473P	Fourteen Mile Creek Tributary 1 to Fourteen Mile Creek Tributary 2 to Fourteen Mile Creek	Lexington County	03/13/2008
08-04-1961P	Long Branch Long Branch Tributary A	Lexington County	03/05/2008
10-04-0151P	Tributary to Red Bank Creek	Lexington County	05/21/2010
11-04-3465P	Kinley Creek Unnamed Tributary to Kinley Creek	City of Columbia; Lexington County	06/13/2011

Numerous tributaries of previous studies and LOMRs have had stream names revised for better conformity throughout the county. These changes include the following:

Flooding Source Effective Name	Flooding Source New Name
Unnamed Tributary to Beach Creek	Beach Creek Tributary
Tributary CR-1	Congaree River Tributary 1
Tributary CR-1-1	Congaree River Tributary 1-1
Tributary R-2	Rawls Creek Tributary 2
Tributary SM-2	Six Mile Creek Tributary 2
Tributary SM-3	Six Mile Creek Tributary 3
Tributary SM-4	Six Mile Creek Tributary 4
Tributary SM-5	Six Mile Creek Tributary 5
Tributary SM-6	Six Mile Creek Tributary 6
Tributary SM-7	Six Mile Creek Tributary 7
Tributary No. 1 To Fourteen Mile Creek	Tributary 1 To Fourteen Mile Creek
Tributary No. 2 To Fourteen Mile Creek	Tributary 2 To Fourteen Mile Creek

Floodplain boundaries of streams that were previously studied by approximate, limited detailed, and detailed methods and have not been restudied in this revision have been redelineated using more up-to-date topographic information.

Approximate analyses are used to study those areas having low development potential or minimal flood hazards.

The areas studied by limited detailed methods were selected with moderate priority given to all known flood hazard areas, and areas of projected development and proposed construction.

Limits of detailed studies are indicated on the Flood Profiles (Exhibit 1) and on the FIRM (Exhibit 2). The areas studied by detailed methods were selected with priority given to all known flood hazard areas, and areas of projected development and proposed construction.

The scope and methods of study were proposed to, and agreed upon by, FEMA, SCDNR, and Lexington County.

2.2 Community Description

Lexington County is located in the central region of South Carolina, immediately southwest of the City of Columbia. It is bordered by Newberry County to the northwest, Saluda County to the West, Aiken County to the southwest, Orangeburg County to the south, Calhoun County to the southeast,

and Richland County to the northeast. The county encompasses an area of 757 square miles which includes 23 square miles of water. According to U.S. Census Bureau figures the population has increased from 216,014 in 2000 to 262,391 in 2010, a 21.5% increase (Reference 5).

The eastern boundary of Lexington County is formed by the Congaree River, and a portion of the Saluda River. The southwestern county boundary is formed by the Chinquapin Creek and North Fork Edisto River. Lake Murray inundates a large portion of area in the northern tip of the county. Twelve Mile Creek and Fourteen Mile Creek are significant tributaries to the Saluda River, which flow through the Town of Lexington. Congaree Creek is a significant tributary to the Congaree River, flowing through the City of Cayce, and Towns of Pine Ridge and South Congaree. Six Mile Creek is also an important stream passing through the City of Cayce and the Town of Springdale, before its confluence with Congaree Creek.

Most of the urbanized area of Lexington County is located on the east side of the county in the vicinity of the Cities of Columbia, West Columbia, and Cayce and the Towns of Irmo, Pine Ridge, South Congaree, and Springdale. Except for a few small urban areas in the vicinity of the Town of Batesburg-Leesville near the western county boundaries, the remainder of the land is wooded or used for agricultural purposes.

Soils in Lexington County are generally excessively drained silty sands, and loams, with local deposits of rock and gravels. In the creek bottoms, soils generally consist of alluvial sand and silt blanketed with finer (clay) soils with local deposits of sands and gravels.

The climate of central South Carolina is temperate. Average monthly temperatures range from 84 degrees Fahrenheit (°F) in the summer to 39°F in the winter. Average annual precipitation for the region is 46 inches. The precipitation is fairly evenly distributed throughout the year, but approximately forty percent of the annual rainfall can be expected to occur during the period of June through October (Reference 6).

The Appalachian Mountains to the west block many cold air masses arriving from the northwest, making the winters somewhat milder (Reference 7). The mountains also cause the areas leeward of the mountains to experience slightly higher temperatures and decreased precipitation compared to surrounding areas.

2.3 Principal Flood Problems

The past history of flooding on the streams in Lexington County indicates that flooding may occur during any season of the year. However, floods on the larger streams, the Congaree, North Fork Edisto, and Saluda Rivers, are more likely to occur from June through October as a result of tropical storms

(hurricanes).

The three worst floods on the Congaree and Saluda Rivers occurred in August 1908, August 1928, and October 1929. Peak discharges for these events were 364,000 cubic feet per second (cfs), 311,000 cfs, and 303,000 cfs respectively for U.S. Geological Survey (USGS) gage Congaree River at Columbia, SC (02169500) (Reference 8).

The maximum stage recorded for Congaree River at Columbia, SC (02169500) was 152.8 feet, National Geodetic Vertical Datum of 1929 (NGVD29), which occurred during the August 1908 flood.

The 1928 flood was caused by two tropical storms which passed over the Santee Basin. The first storm, which occurred August 10 and 11, was centered across the middle of the Carolinas, and did not raise the rivers to excessive heights. The second storm, which took place on August 15 and 16, resulted in major flooding throughout the basin. The Congaree River at Columbia, SC (02169500) reached 37.5 feet on August 18, which was 2.3 feet below the high-water mark on August 27, 1908.

The two tropical storms of September 22 and 27, and October 1 and 2, 1929, produced floods of exceptional severity over the Saluda and Broad River watersheds, establishing new high-water marks on some of the tributaries. At Broad River at Blair, SC (02160750), a gage height of 38.5 feet was reached, 7.5 feet above the recorded stage of August 1908. The Congaree River at Columbia, SC (02169500) reached a stage of 37.1 feet, 2.7 feet below the high-water mark of August 1908. The Saluda Dam was near completion and Lake Murray was in the process of being filled when the 1929 flood occurred. The storage provided by the partially filled lake resulted in considerable reduction of the flood peak at Saluda River Below Lake Murray Dam near Columbia, SC (02168504). If the lake storage had not been available, the 1929 flood stage at Columbia would probably have exceeded the previous high-water mark of August 1908. A number of bridges on tributaries of the Broad and Saluda Rivers were washed out; some state and county highways were closed to traffic for several days; and a number of mills and small hydro-electric plants were put out of commission for short periods.

The October 1929 flood was the maximum known flood on the Saluda River. At Saluda River near Columbia, SC (02169000), located approximately 2 miles above the mouth, the river reached an elevation of 164.7 feet NGVD, and the estimated peak discharge was 67,000 cfs. If the same flood occurred under existing conditions, the peak stage and discharge would be much higher. For this reason, a meaningful comparison between the 1929 flood and computer flood frequency-discharge relationships for the Saluda River cannot be made. The computed 100-year frequency stage and discharge at Saluda River near Columbia, SC (02169000) are 168.9 feet NGVD and 105,000 cfs, respectively.

Stream gage data have been collected at Congaree Creek at Cayce, SC (02169550) since 1959 (Reference 9). During the period of record, the maximum flood (October 1959) reached a peak discharge of 1,840 cfs and crested at 134.9 feet NGVD. This flood had an approximate return frequency of 18 years. The estimated 100-year flood at the same location has a peak discharge of 14,800 cfs and a crest stage of 143.5 feet NGVD. The October 1959 flood caused only a small amount of damage in Lexington County because there was little development in the floodplain at that time.

The flood of record, which occurred in October 1976, had a peak discharge at Congaree River at Columbia, SC (02169500) of 155,000 cfs and approximated a 10-year flood.

There is no information available on past flood history for Six Mile Creek and its tributaries in the Town of Springdale. Since the drainage areas are small and there is a considerable amount of urban development in the basin, it is reasonable to assume that floods can occur at any time during the year from local thunderstorms. Following intense rainfall over the basin, floods will rise and fall swiftly.

There is very little historic flood data available on Twelve Mile Creek. Interviews with local residents indicated that the dams forming Gibson Pond and Lexington Mill Pond failed during a flood in April 1936. There was no development in the reach between the ponds, but a store and several cabins located below Lexington Mill Pond were washed away. Both dams were reconstructed and no failures have occurred for the past 40 years. Reconstruction of Lexington Mill Pond Dam included a manually operated emergency spillway, a feature which the original structure did not have. These gates can be opened to lower the pond when flood warnings are received. Stage-discharge data on other streams in the study area were not available.

2.4 Flood Protection Measures

Floods in the study area may be affected by operation of two large reservoirs on the Saluda River. Lake Greenwood is formed by Buzzards Roost Dam, which was completed in 1940, and is owned by Greenwood County. The dam and powerhouse were operated, under lease, by Duke Energy from 1966 to 2006. Santee Cooper is the current operator of the transmission lines, dam and powerhouse. Lake Greenwood, located at river mile 60 has a surface area of approximately 11,400 acres at full pool elevation of 440 feet mean sea level. Saluda Dam (commonly-Lake Murray Dam, officially-Dreher Shoals Dam) was completed in 1930 by Lexington Water Power Company and is located about 12 miles above the mouth of the Saluda River, or 10 miles west of the City of Columbia and forms Lake Murray. South Carolina Electric & Gas Company currently owns and operates Saluda Dam, with the Saluda Hydroelectric Plant.

At full pool elevation, 358.8 feet North American Vertical Datum of 1988 (NAVD88), it has a surface area of approximately 51,000 acres.

Both reservoirs were initially constructed for hydroelectric power generation and are subject to regulations prescribed by the Federal Energy Regulatory Commission (FERC). Both of these dams are currently operated to produce hydroelectric power.

A levee exists along the east bank of the Congaree River that provides Richland County with some degree of protection against flooding, but none for Lexington County. FEMA specifies that all levees must meet the criteria of NFIP regulations Section 65.10 to be considered a safe flood protection structure. The criteria used to evaluate protection against the 1-percent-annual-chance flood are 1) adequate design, including freeboard, 2) structural stability, and 3) proper operation and maintenance. It has been determined that the levee along the Congaree River does not meet these requirements. Therefore, since the levee does not meet all of the requirements, the levee cannot be certified as providing protection against the 1-percent-annual-chance flood.

Non-structural measures of flood protection have been implemented by Lexington County to aid in the prevention of future flood damage. These are in the form of subdivision regulations which control construction within flood hazard areas (Reference 10).

3.0 ENGINEERING METHODS

For the flooding sources studied by limited detailed and detailed methods in the community, standard hydrologic and hydraulic study methods were used to determine the flood hazard data required for this study. Flood events of a magnitude that are expected to be equaled or exceeded once on the average during any 10-, 25-, 50-, 100-, or 500-year period (recurrence interval) have been selected as having special significance for floodplain management and for flood insurance rates. These events, commonly termed the 10-, 25-, 50-, 100-, and 500-year floods, have a 10-, 4-, 2-, 1-, and 0.2-percent chance, respectively, of being equaled or exceeded during any year. Although the recurrence interval represents the long-term, average period between floods of a specific magnitude, rare floods could occur at short intervals or even within the same year. The risk of experiencing a rare flood increases when periods greater than 1 year are considered. For example, the risk of having a flood that equals or exceeds the 1-percent-annual-chance flood in any 50-year period is approximately 40 percent (4 in 10); for any 90-year period, the risk increases to approximately 60 percent (6 in 10). The analyses reported herein reflect flooding potentials based on conditions existing in the community at the time of completion of this study. Maps and flood elevations will be amended periodically to reflect future changes.

3.1 Hydrologic Analyses

Hydrologic analyses were carried out to establish peak discharge-frequency relationships for each flooding source studied affecting Lexington County, including incorporated communities, and unincorporated areas.

Pre-countywide Analyses

Each community within Lexington County, except for the Towns of Batesburg-Leesville, Gilbert, Pelion, and Swansea had a previously printed FIS report. The hydrologic analyses described in those reports have been compiled and are summarized below.

Several methods of computation were used to compute peak discharges due to variations in drainage area size, type of development within the watersheds, and availability of stream gage data for the streams selected for detailed study.

Discharge-frequency relationships for the Congaree and Saluda Rivers were derived using the log-Pearson Type III distribution method based on stream gage records collected at USGS stream gage stations (Reference 11, & 12).

Table 10 – USGS Stream Gage Stations

Gage ID	Flooding Source and Location	Years of Record
02161500	Broad River at Richtex, SC	52
02169000	Saluda River near Columbia, SC	52
02169500	Congaree River at Columbia, SC	86

Records have been collected on the Congaree River at Columbia, SC (02169500) gage since 1891 and on the Saluda River near Columbia, SC (02169000) gage and the Broad River at Richtex, SC (02161500) gage since 1925. The construction of Saluda Dam in 1929 altered the flood situation at both gaging stations 02161500 and 02169000.

Maximum operating pool level of Lake Murray, as regulated by FERC is 360 feet (NGVD29). When inflow during major floods requires temporary storage above maximum operating pool level, releases are made through spillway gates to augment discharges through power turbines in order to lower the reservoir to required maximum pool level as soon as possible. During this operation, spillway gates are opened gradually until the lake level begins to recede. As long as the reservoir level continues to rise gate openings will be increased until all six spillway gates are wide open. This type of operation attempts to keep

outflow approximately equal to inflow without allowing the reservoir to rise to a dangerous level. If the reservoir happens to be below normal operating level, prior to a flood occurrence, some of the floodwater will be stored, resulting in a reduction of peak discharges downstream.

The chance of incidental flood control storage is greater for minor floods than for major events; therefore, it was assumed that stream-flow records collected on the Saluda River near Columbia, SC (02169000) and the Congaree River at Columbia, SC (02169500) could be used, without adjustments, to determine discharge-frequency relationships for floods up to 10-year frequency at both stations. In order to establish the upper end of the discharge-frequency curves, it was necessary to adjust recorded flood discharges which were affected by coincidental flood control storage. This was accomplished by applying methods based on the hydrologic equations utilizing peak discharge and mean discharge information supplied by the USGS and South Carolina Electric & Gas Company (Reference 13, & 14). The adjustments provided a homogeneous set of data which was used as a basis for probability studies to establish the portion of the discharge-frequency curves from the 50- to 500-year frequencies at both stations. Smooth transitions were drawn between the upper and lower frequency curves for both stations.

Six Mile Creek discharge-frequency relationships were developed using methods prescribed in a USGS open-file report, and the results were checked against the results of a rainfall-runoff model developed during a USACE floodplain information study report (Reference 15, & 16).

Discharge-frequency determinations for Big Branch, Kinley Creek, Koon Branch, Rawls Creek, Senn Branch, Six Mile Creek, Stoop Creek, Tributary CR-l, Tributary CR-l-l, Tributary K-2, Tributary R-2, Tributary SM-3, and Tributary SM-5, were computed using USGS urban runoff formulas contained in an open-file report (Reference 15).

For Rawls Creek in the Town of Irmo, the discharge at the upstream corporate limit was estimated from regional regression equations (Reference 17). Adjustments to the discharges were made for future urbanization (Reference 15, 18, & 19).

Discharges were not determined for Kinley Branch, Tributary F-l, Tributary R-l, Tributary SM-4, Tributary SM-6, Tributary SM-7, Tributary TM-l, Tributary TM-2, Tributary TM-3, and Tributary TM-3-1 which were studied by approximate methods. Flood boundaries for these streams were estimated based on information developed for detailed study reaches in the same area.

Additional data used to confirm frequency curves developed by the methods described above included a Standard Project Flood developed for the Congaree River at Columbia, SC (02169500) (Reference 20); and a Standard Project

Flood developed by the USACE, Savannah District, during preparation of the Detailed Design Memorandum for the Cooper River Rediversion Project (Reference 21).

Revised Analyses for the July 17, 1995, Countywide FIS

Peak discharges for Tributary K-2, Tributary SM-2, and Yost Creek were computed using the USACE HEC-1 Flood Hydrograph Package (HEC-1) rainfall-runoff model (Reference 22). Rainfall values for storms having recurrence intervals of 10-, 50-, and 100-years were obtained using the U.S. Weather Bureau Technical Paper No. 40 (TP-40) publication (Reference 23). The rainfall for the 500-year storm was determined using graphical methods. Index rainfall values were adjusted for drainage area for input to the model. The Soil Conservation Service (SCS) unit hydrograph procedures were used to establish rainfall-runoff relationships. Topographic maps and field inspections were used to determine the hydrologic characteristics of the stream basins included in this study (Reference 24). SCS unit hydrograph parameters were determined from the basin characteristics using the guidelines outlined in SCS Technical Release No. 55 (TR-55) (Reference 19). Using the unit hydrograph parameters and rainfall data obtained as described above, stream basins were modeled with HEC-1 to determine peak discharges along the streams for floods of the selected recurrence intervals.

Hydrologic analyses were performed to determine peak discharge-frequency relationships for the streams included in this special study. Peak discharges for Senn Branch were obtained from a detailed hydrologic study of the watershed, which was conducted by the Lexington County Department of Planning and Development (DPD), in cooperation with the University of South Carolina. This study utilized the DRAIN:EDGE numerical model to simulate the rainfall-runoff process. The Lexington County DPD utilized the Geographical Information System to determine basin parameters for input to the DRAIN:EDGE model. Peak discharges for the 10-, 50-, and 100-year floods were taken from the Lexington County DPD study. Peak discharges for the 500-year flood were obtained graphically through extrapolation of the probability distribution.

Peak discharges for Tributary SM-3 were determined by using the computed discharges from the effective FIS. Drainage area relationships were used to adjust the peak discharges for changes in runoff due to changes in the watershed area.

The revised hydrologic analyses for Stoop Creek were performed using HEC-1 to establish peak discharge-frequency relationships for floods of the selected recurrence intervals. The discharges for Stoop Creek were developed from the hydrologic analyses from the February 4, 1987, FIS for the City of Columbia (Reference 25).

Revised Analyses for the February 9, 2000, Countywide FIS

The revised hydrologic analyses were performed using HEC-1. The SCS dimensionless unit hydrograph was used as the method to calculate the hydrograph for each sub-basin. The Muskingum method was used for the routing methodology. The raw data for the drainage areas, curve numbers, lag and routing times were obtained from USGS Quadrangle Maps (Reference 26, & 27). The hypothetical storm information was obtained from TP-40.

The detailed study areas were divided into two categories, the Twelve Mile Creek watershed, and the Congaree Creek watershed. The Twelve Mile Creek watershed contains Fourteen Mile Creek and Twelve Mile Creek. The Congaree Creek watershed contains First Creek, Second Creek, Bear Creek, Hunt Branch, Lick Fork Branch, Red Bank Creek, Savana Branch and Congaree Creek. The revised discharges on Fourteen Mile Creek and Twelve Mile Creek reasonably compared with the discharges in the effective study, so the models were extended. The Congaree Creek discharges did not compare well with those used in the effective study, so the model was replaced using the revised discharges.

Within the studied area there are two USGS stream-flow gages. The gage station located at Congaree Creek at Cayce, SC (02169550) was in operation from 1959 to 1980. The gage station located at Savana Branch near Cayce, SC (02169540) was in operation from 1968 to 1989. Due to the poor reliability of such a short period of record, the hydrologic models for studied watersheds were calibrated to historical floods using the hydraulic models and historical high-water elevations along studied streams.

The Lake Murray stillwater elevation of 362.5 feet NGVD29 was computed using HEC-1.

Revised Analyses for the February 20, 2002, Countywide FIS

The discharges for the Congaree River were developed by analyzing two major contributing watersheds: the Saluda River/Lake Murray watershed and the Broad River watershed. Peak flow records at Congaree River at Columbia, SC (02169500) were analyzed following Bulletin 17B guidelines (Reference 28). These peak discharges were transposed south to the corporate boundary between Lexington County and Calhoun County (Reference 29). The Saluda Dam construction started in the fall of 1927, and was completed in 1930. The USGS gaging station Congaree River at Columbia, SC (02169500) provides a uniform data set from water year 1931 to the present date. In water years 1928 and 1930, during construction of the Saluda Dam, two large floods occurred. In addition, there are records of annual maximum flows on the USGS gaging station Broad River at Richtex, SC (02161500) from 1925 on, occurring under uniform basin conditions. The peak flow records from these gages are also incorporated into

the analysis. The flood discharges for Broad River and Saluda River have not been revised as part of this restudy.

This Countywide Analysis

For this report streams which were studied were divided into four classifications; approximate, enhanced approximate, limited detailed, and detailed, based on their method of study.

Peak flood discharges for the 1-percent-annual-chance storm event for all streams studied by approximate and enhanced approximate methods were determined using USGS regression equations for South Carolina, described in USGS Scientific Investigations Report (SIR) 2009-5156, and SIR 2004-5030. Urban regression equations were used in basins that were considered to be 'urbanized', which applies to any basin with impervious area being greater than or equal to 10% of the total basin area.

The peak flood discharges for the 10-, 4-, 2-, 1-, and 0.2-percent-annual-chance storm events for all streams studied by limited detailed methods were determined using the same USGS regression equations described above for approximate and enhanced approximate. Urban regression equations were used in basins that were considered to be 'urbanized', which applies to any basin with impervious area being greater than or equal to 10% of the total basin area.

Peak flood discharges for the 10-, 4-, 2-, 1-, and 0.2-percent-annual-chance storm events for all streams studied by detailed methods were determined using USACE Hydrologic Engineering Center – Hydrologic Modeling System (HEC-HMS) version 3.5 and Bulletin 17B analysis guidelines. The regression equations were not used for these streams due to the presence of significant impoundments, specifically on Kinley Creek, which could impact the accuracy of regression discharges.

Drainage areas along streams were determined using a flow accumulation grid as part of a digital elevation model (DEM) developed from the Light Detection and Ranging (LiDAR) data collected and processed as part of this study. Flow points along stream centerlines were calculated using the regression equations in conjunction with accumulated area for approximately every 10 percent increase in flow along a particular stream.

There are multiple gages located throughout Lexington County but gage adjustments for this study were not performed, because the flooding sources studied by limited detailed and detailed methods have extenuating circumstances; Saluda River is a well regulated stream thus gage records are of no use, and Bull Swamp Creek has limited (2-years) recorded data.

A summary of peak discharge-drainage area relationships for streams studied by detailed methods is shown in Table 11, “Summary of Discharges”.

Table 11 – Summary of Discharges

Flooding Source and Location	Drainage Area (Square Miles)	Peak Discharges (Cubic Feet per Second)				
		10% Annual Chance	4% Annual Chance	2% Annual Chance	1% Annual Chance	0.2% Annual Chance
BEACH CREEK						
At Beechwoods Drive	0.89	*	*	*	2,390	*
BEACH CREEK TRIBUTARY						
At mouth	0.11	*	*	*	500	*
BEAR CREEK						
Downstream of Feigles Pond	6.2	225	*	555	793	1,820
BIG BRANCH						
At Fish Hatchery Road	1.0	388	*	744	929	1,485
BROAD RIVER						
At mouth	5,344	121,000	162,000	199,000	241,000	364,000
At USGS Gage Station No. 02161500	4,848	114,000	153,000	188,000	227,000	344,000
CONGAREE CREEK						
Upstream of confluence of Six Mile Creek	121.9	3,460	*	6,880	9,320	18,900
Upstream of U.S. Highway 321	120.1	3,490	*	6,940	9,490	19,600
Upstream of confluence of Savana Branch	108.9	3,380	*	6,670	9,200	19,400
Upstream of confluence of First Creek	72.4	2,640	*	5,250	6,940	13,300
Downstream of confluence of Red Bank Creek	68.4	2,610	*	5,220	6,950	13,500
Upstream of Red Bank Creek	36.5	1,460	*	2,980	4,040	8,230
Downstream of Hunt Pond Dam	35.1	1,450	*	2,970	4,050	8,360
Upstream of Hunt Pond Dam	35.1	1,580	*	3,190	4,220	8,070
At upstream end of Hunt Pond	34.6	1,580	*	3,180	4,200	8,030
Approximately 6,000 feet downstream of Old Orangeburg Road	32.6	1,560	*	3,150	4,170	7,990
Approximately 500 feet upstream of Old Orangeburg Road	27.7	1,510	*	3,060	4,060	7,800
Upstream of confluence of Scouter Branch	15.2	1,170	*	2,360	3,120	5,950
Downstream of Moragne Pond	11.4	1,030	*	2,130	2,830	5,490
Platt Springs Road	5.2	466	*	981	1,310	2,590
CONGAREE RIVER						
At USGS Gage Station No. 02169500	7,850	147,600	197,300	239,400	286,000	414,500

Table 11: Summary of Discharges – continued

Flooding Source and Location	Drainage Area (Square Miles)	Peak Discharges (Cubic Feet per Second)				
		10% Annual Chance	4% Annual Chance	2% Annual Chance	1% Annual Chance	0.2% Annual Chance
CONGAREE RIVER TRIBUTARY 1						
At Railroad	1.96	1,203	*	1,889	2,182	3,059
CONGAREE RIVER TRIBUTARY 1-1						
At mouth	0.38	471	*	799	960	1,334
FIRST CREEK						
At confluence with Congaree Creek	35.5	2,320	*	4,360	5,930	12,100
Approximately 1,900 feet downstream of Dogwood Road	33.9	2,370	*	4,450	6,120	12,800
Approximately 1,900 feet upstream of Dogwood Road	32.1	2,310	*	4,360	6,020	12,800
Upstream of confluence of Second Creek	14.9	1,570	*	2,920	3,760	6,750
Approximately 2,300 feet downstream of Hutto Pond Dam	14.4	1,530	*	2,870	3,700	6,670
Approximately 3,800 feet upstream of Hutto Pond Dam	11.1	1,250	*	2,380	3,090	5,650
Approximately 1,800 feet downstream of Urquhart Pond Dam	8.8	1,220	*	2,270	2,930	5,270
Approximately 2,000 feet downstream of Woodtrail Drive	4.5	455	*	948	1,270	2,500
At Woodtrail Drive	4.2	419	*	889	1,190	2,360
FOURTEEN MILE CREEK						
At Park Road	4.5	1,160	*	1,830	2,230	3,530
Upstream of confluence of Long Branch	1.9	113	*	301	432	990
At Wise Ferry Road	0.8	93	*	221	306	690
Downstream of confluence of Tributary No. 2 to Fourteen Mile Creek	0.1	*	*	*	48	*
HUNT BRANCH						
Approximately 6,200 feet upstream of Feigles Pond	2.3	125	*	295	412	895

Table 11: Summary of Discharges – continued

Flooding Source and Location	Drainage Area (Square Miles)	Peak Discharges (Cubic Feet per Second)				
		10% Annual Chance	4% Annual Chance	2% Annual Chance	1% Annual Chance	0.2% Annual Chance
KINLEY CREEK						
Approximately 2,040 feet downstream of St. Andrews Road	7.12	2,944	3,915	4,566	6,064	9,283
Immediately upstream of confluence of Kinley Creek Tributary 2	3.99	1,741	2,285	2,721	3,207	4,884
Immediately downstream of Beaver Dam Road	1.85	*	*	*	1,711	*
KINLEY CREEK TRIBUTARY 1						
At mouth	0.45	466	627	767	922	1,338
KINLEY CREEK TRIBUTARY 2						
At mouth	1.52	1,290	1,703	2,116	2,604	3,778
KOON BRANCH						
At mouth	1.5	960	*	1,560	1,820	2,590
At southern corporate limits of the Town of Irmo	0.5	510	*	870	1,040	1,450
LICK FORK BRANCH						
At confluence with Red Bank Creek	4.4	589	*	1,210	1,610	3,110
Downstream of Lown Pond	3.1	372	*	837	1,140	2,360
At Kitti Wake Drive	1.4	229	*	487	651	1,280
LONG BRANCH						
At confluence with Fourteen Mile Creek	2.2	*	*	*	2,331	*
Immediately upstream of confluence of Long Branch Tributary A	1.6	*	*	*	1,628	*
LONG BRANCH TRIBUTARY A						
At confluence with Long Branch	0.6	*	*	*	926	*
RAWLS CREEK						
At mouth	10.3	2,930	*	4,310	4,810	6,880
At southwest corporate limits of the Town of Irmo	3.2	1,540	*	2,380	2,720	3,840
Upstream of confluence of Rawls Creek Tributary 2	1.9	1,080	*	1,730	2,010	2,870
At the Lexington–Richland County Boundary	1.89	1,060	1,499	1,879	2,242	3,366

Table 11: Summary of Discharges – continued

Flooding Source and Location	Drainage Area (Square Miles)	Peak Discharges (Cubic Feet per Second)				
		10% Annual Chance	4% Annual Chance	2% Annual Chance	1% Annual Chance	0.2% Annual Chance
RAWLS CREEK TRIBUTARY 2						
At mouth	1.2	820	*	1,340	1,580	2,250
At the Lexington–Richland County Boundary	1.23	988	1,320	1,622	1,886	2,789
RED BANK CREEK						
Upstream of confluence with Congaree Creek	31.9	1,410	*	2,920	3,880	7,520
Downstream of Durham Pond Dam	31.2	1,390	*	2,900	3,860	7,510
Upstream of Durham Pond Dam	31.2	1,410	*	2,920	3,890	7,580
At upstream end of Durham Pond	26.7	1,300	*	2,740	3,660	7,210
Approximately 400 feet downstream of Old Orangeburg Road	23.1	1,220	*	2,580	3,460	6,840
Downstream of Crystal Lake Dam	20.6	1,160	*	2,470	3,330	6,630
Upstream of Crystal Lake Dam	20.6	1,260	*	2,680	3,560	6,900
At upstream end of Crystal Lake	18.0	1,210	*	2,570	3,430	6,710
At Saxe-Gotha Millpond Dam (busted)	17.5	1,190	*	2,540	3,390	6,660
Approximately 3,200 feet upstream of Saxe-Gotha Millpond Dam	16.2	1,150	*	2,470	3,290	6,440
Upstream of confluence of Turkey Creek	8.6	909	*	2,090	2,780	5,420
Approximately 2,200 feet downstream of Private Dam (RBC11)	5.2	786	*	1,810	2,390	4,550
Downstream of Private Dam (RBC11)	4.5	751	*	1,740	2,300	4,370
At Calks Ferry Road	2.2	363	*	741	982	1,890
SALUDA RIVER						
At USGS Gage Station No. 02169000	2,520	29,600	39,400	48,300	58,600	89,900
At USGS Gage Station No. 02169000 (or slightly upstream of the City of Columbia corporate limits)	2,510	32,000	*	90,000	105,000	145,000
SAVANA BRANCH						
At confluence with Congaree Creek	7.4	536	*	1,170	1,670	3,800
Downstream of Columbia Metropolitan Airport runway culvert	6.7	507	*	1,130	1,610	3,660
Downstream of Pitts Lake Dam	6.2	492	*	1,110	1,600	3,740
Upstream of Pitts Lake Dam	6.2	677	*	1,230	1,630	3,100

Table 11: Summary of Discharges – continued

Flooding Source and Location	Drainage Area (Square Miles)	Peak Discharges (Cubic Feet per Second)				
		10% Annual Chance	4% Annual Chance	2% Annual Chance	1% Annual Chance	0.2% Annual Chance
SAVANA BRANCH (continued)						
Approximately 2,500 feet upstream of Platt Springs Road	4.2	543	*	1,020	1,390	2,900
Approximately 2,550 feet upstream of Platt Springs Road	3.1	456	*	799	1,110	2,390
Downstream of Bradley Drive Dam	1.8	136	*	477	705	1,810
Upstream of Bradley Drive Dam	1.8	383	*	723	931	1,670
Approximately 2,900 feet upstream of Bradley Drive Dam	0.7	146	*	279	360	650
SECOND CREEK						
At confluence with First Creek	16.7	2,280	*	4,320	5,990	12,770
Approximately 3,200 feet downstream of Gator Road	14.6	644	*	1,320	2,130	6,490
Downstream of Private Dam (FC15)	11.3	451	*	1,030	1,640	4,760
Upstream of Private Dam (FC15)	11.3	594	*	1,320	1,810	3,730
Approximately 1,900 feet upstream of Private Dam (FC15)	9.1	465	*	1,040	1,440	3,090
SENN BRANCH						
At mouth	2.1	1,270	*	2,010	2,320	3,290
At Ephrata Drive	1.25	742	*	1,150	1,355	1,890
At Highway 378	0.67	246	*	410	498	725
At Dew Drop Lane	0.47	156	*	275	324	510
At Hebron Drive	0.36	123	*	210	254	390
SIX MILE CREEK						
At mouth	13.5	2,830	*	4,300	4,840	7,180
Upstream of Six Mile Creek Tributary 3 confluence	8.8	2,314	*	3,550	4,023	5,910
Upstream of Six Mile Creek Tributary 5 confluence	5.9	1,910	*	2,961	3,378	4,918
Upstream of Six Mile Creek Tributary 6 confluence	4.6	1,580	*	2,510	2,890	4,240
Below Southern Railway	3.83	1,410	*	2,260	2,620	3,840

Table 11: Summary of Discharges – continued

Flooding Source and Location	Drainage Area (Square Miles)	Peak Discharges (Cubic Feet per Second)				
		10% Annual Chance	4% Annual Chance	2% Annual Chance	1% Annual Chance	0.2% Annual Chance
SIX MILE CREEK TRIBUTARY 2						
At mouth	0.94	298	*	477	567	766
At breached dam	0.74	279	*	448	533	723
SIX MILE CREEK TRIBUTARY 3						
At mouth	2.5	1,260	*	2,000	2,310	3,300
At Edmund Road (Highway 302)	2.01	1,130	*	1,793	2,062	2,959
At Railroad Bridge	1.49	973	*	1,544	1,776	2,548
At Biloxi Square	1.21	247	*	715	1,101	1,936
SIX MILE CREEK TRIBUTARY 5						
At mouth	1.3	826	*	1,362	1,608	2,304
SIX MILE CREEK TRIBUTARY 6						
At Hollydale Drive	0.6	*	*	*	145	*
STOOP CREEK						
At mouth	4.29	1,642	*	1,973	2,203	3,141
At Interstate Highway 20	3.96	2,115	*	2,995	3,483	4,664
At Interstate Highway 26	3.29	1,699	*	2,450	2,831	3,763
TRIBUTARY TO FOURTEEN MILE CREEK						
At confluence with Fourteen Mile Creek	0.6	410	*	627	723	930
Approximately 1,890 feet upstream of confluence with Fourteen Mile creek	0.4	266	*	403	462	600
TRIBUTARY TO RED BANK CREEK						
At mouth	1.62	*	*	*	1,152	*
At Interstate-20	1.10	*	*	*	568	*
TRIBUTARY 1 TO FOURTEEN MILE CREEK						
Approximately 1,800 feet upstream of confluence with Fourteen Mile Creek	0.2	*	*	*	88	*
TRIBUTARY 2 TO FOURTEEN MILE CREEK						
Approximately 1,800 feet upstream of confluence with Fourteen Mile Creek	0.04	*	*	*	28	*

Table 11: Summary of Discharges – continued

Flooding Source and Location	Drainage Area (Square Miles)	Peak Discharges (Cubic Feet per Second)				
		10% Annual Chance	4% Annual Chance	2% Annual Chance	1% Annual Chance	0.2% Annual Chance
TWELVE MILE CREEK						
Downstream of Gibson Pond Dam	31.0	1,220	*	2,490	3,260	6,050
Upstream of Gibson Pond Dam	31.0	1,370	*	2,590	3,330	5,970
Upstream end of Gibson Pond	30.1	1,360	*	2,570	3,300	5,920
Downstream of confluence of Boggy Branch	28.9	1,340	*	2,570	3,300	5,950
Downstream of Barr Lake Dam	27.1	1,300	*	2,500	3,220	5,800
Upstream end of Barr Lake	25.9	1,330	*	2,490	3,220	5,830
Downstream of confluence of Hogpen Branch	22.2	1,240	*	2,380	3,090	5,700
Upstream of confluence of Hogpen Branch	19.5	1,130	*	2,190	2,860	5,330
Downstream of confluence of Long Creek	16.2	1,050	*	2,040	2,670	5,010
Downstream of Crout Pond Dam	7.7	503	*	1,510	1,980	3,670
Upstream of Crout Pond Dam	7.7	847	*	1,570	2,020	3,630
At the Town of Gilbert corporate limits	4.3	641	*	1,220	1,580	2,880
UNNAMED TRIBUTARY TO TWELVE MILE CREEK						
At mouth	2.85	*	*	*	3,633	*
At Interstate-20	2.61	*	*	*	3,469	*
YOST CREEK						
At mouth	1.21	533	*	775	886	1,104
Approximately 1,300 feet upstream of Coldstream Drive	0.75	467	*	683	783	975
Approximately 115 feet downstream of Lincreek Drive	0.38	427	*	636	732	894

* Data Not Available

Flooding sources with stillwater elevations established are displayed in the following table.

Table 12 – Summary of Stillwater Elevations

Flooding Source and Location	Elevation (feet NAVD88)				
	10% Annual Chance	4% Annual Chance	2% Annual Chance	1% Annual Chance	0.2% Annual Chance
SALUDA RIVER Lake Murray	*	*	*	361.7	*
SIX MILE CREEK TRIBUTARY 6 Parrish Pond approximately 100 feet upstream of Hollydale Drive	*	*	*	231.3	*

*** – denotes data unavailable**

3.2 Hydraulic Analyses

Analyses of the hydraulic characteristics of flooding from the sources studied were carried out to provide estimates of the elevations of floods of the selected recurrence intervals. Users should be aware that flood elevations shown on the FIRM represent rounded whole-foot elevations and may not exactly reflect the elevations shown on the Flood Profiles or in the Floodway Data Tables in the FIS report. Flood elevations shown on the FIRM are primarily intended for flood insurance rating purposes. For construction and/or floodplain management purposes, users are cautioned to use the flood elevation data presented in this FIS report in conjunction with the data shown on the FIRM.

Locations of selected cross-sections used in the hydraulic analyses are shown on the Flood Profiles (Exhibit 1). For stream segments for which a floodway was computed (Section 4.2), selected cross-section locations are also shown on the FIRM.

Flood profiles were drawn showing the computed water-surface elevations for floods of the selected recurrence intervals.

Along certain portions of streams, a profile base line is shown on the maps to represent channel distances as indicated on the Flood Profiles and Floodway Data Tables.

The hydraulic analyses for this study were based on unobstructed flow. The flood elevations shown on the profiles are thus considered valid only if hydraulic structures remain unobstructed, operate properly, and do not fail.

Pre-countywide Analyses

Each community within Lexington County, except for the Towns of Batesburg Leesville, Gilbert, Pelion, and Swansea, had a previously printed FIS report. The hydraulic analyses described in those reports have been compiled and are summarized below.

Cross-sections for the backwater analyses were obtained from field surveys. Cross-sections were located at close intervals above or below bridges and culverts in order to compute the significant backwater effects of these structures.

All bridges, dams, and culverts were surveyed to obtain elevation data and structural geometry.

Additional cross-section information for the Towns of Irmo, Lexington, Pine Ridge, South Congaree, and Springdale was obtained by field surveys and topographic maps at a scale of 1"=200', with a contour interval of 5 feet (Reference 24). Cross-section information for a portion of Rawls Creek in the Town of Irmo was obtained from a topographic map at a scale of 1:600 with a contour interval of 1 foot (Reference 30).

Water-surface elevations of floods of the selected recurrence intervals were computed using the USACE HEC-2 step-backwater computer program (Reference 31).

Starting water-surface elevations were computed using the slope/area method for Twelve Mile Creek, within the Town of Lexington. In the Town of Irmo, starting water-surface elevations for Rawls Creek and Koon Branch were obtained from hydraulic studies conducted on the lower reaches of these streams. The starting water-surface elevations for all other streams studied by detailed methods were determined by coincident peak.

Channel and overbank roughness factors (Manning's 'n') used in the hydraulic computations were chosen by engineering judgment and were based on field observations of the streams and floodplain areas. In the City of Cayce, the Towns of Pine Ridge and South Congaree, and the unincorporated areas, the acceptability of assumed hydraulic factors, cross-sections, and hydraulic structure data was checked by computations which duplicated historic floodwater profiles on streams for which historic data were available. For the Towns of Irmo, Lexington, and Springdale, no past flood information was available; therefore, hydraulic models could not be calibrated with actual stage-discharge data. However, the coefficients used and the results obtained compared favorably with calibrated hydraulic model results on other streams in the same area.

For the unincorporated areas, elevations for approximately studied areas were determined based on drainage area, streambed slope, normal depth calculations, topographic maps, and comparisons with similar streams studied by detailed methods (Reference 26, 27, 32, & 33).

The hydraulic analyses for the unincorporated areas considered possible failure of the dams at Gibson Pond and Lexington Mill Pond on Twelve Mile Creek. Interviews with local residents produced information indicating that both of these structures failed during a flood in April 1936. Several structures located in the floodplain below Lexington Mill Pond were washed away by the surge. No frequency-discharge or elevation data are available on the 1936 flood. Both dams were rebuilt, and the Lexington Mill Pond Dam reconstruction included a gated spillway which can be manually operated to lower the pond level in the event of a flood. The Lexington Mill Pond Dam, with a head differential of 22 feet, appears to be in good condition, and gates appear to be operable. Gibson Pond Dam is not in good condition, but it has a head differential of only 9.5 feet. Since reconstruction following the 1936 flood, neither dam has failed.

Approximate methods based on empirical model study results were used to obtain estimates of the effect of total instantaneous failure of both dams at the time of the 100-year flood peak The results indicated that the additional surge from Gibson Pond would raise the natural 100-year flood peak approximately 4 feet between Gibson Pond and Lexington Mill Pond. The surge from Lexington Mill Pond would raise the natural 100-year flood crest immediately below the dam by approximately 10 feet. These calculations were designed to determine the worst situation that could occur during a 100-year flood to provide upper limits for engineering judgment decisions. On the other hand, if ample flood warnings were received in time for the Lexington Mill Pond gate to be opened and the pond drawn down, the natural 100-year flood crest below Lexington Mill Pond could be reduced significantly.

If one or both of the dams break during a major flood, the break is likely to be partial and occur in several stages. The break or breaks may occur before, during, or after the flood crest, or in various stages during the entire flood. The flood gate at Lexington Mill Pond may or may not be opened in time to provide relief. If the gate is not opened, and the Lexington Mill Pond does not fail, the flood below the dam will be equivalent to a flood under natural conditions (with no dam). These factors and the fact that no failures have occurred since 1936, were considered in formulating a reasonable basis for floodplain management and flood insurance rates in the floodplain of Twelve Mile Creek.

For determination of flood elevations on Twelve Mile Creek it was assumed that neither Gibson Pond Dam nor Lexington Mill Pond Dam will fail, and that there will be no reduction in flood elevations as a result of natural attenuation

or manipulation of the spillway gate at Lexington Mill Pond. Inflow into the system will be equal to outflow.

Revised Analyses for the July 17, 1995, Countywide FIS

Cross-sections for the backwater analyses of all detailed study reaches were obtained by field surveys and information obtained from topographic maps at a scale of 1"=200' with a contour interval of 5 feet (Reference 34). Updated information was obtained from orthophoto/topographic maps, dated March 1989, at a scale of 1"=200', with a 2-foot contour interval. Bridges, dams, and culverts were field checked to obtain elevation data and structural geometry.

Additional cross-sections for Stoop Creek were determined using information obtained from topographic maps at a scale of 1"=200', with a contour interval of 5 feet (Reference 24).

Water-surface elevations of floods of the selected recurrence intervals were computed using the USACE HEC-2 step-backwater computer program (Reference 35). Starting water-surface elevations for each stream restudied or revised by detailed methods were determined using the slope/area method.

The calculations for Twelve Mile Creek were based on Corley Mill Dam not failing and operating at maximum capacity level. According to the owner of the dam, the dam is maintained at full level annually from the months of November through April to flood the region upstream of Corley Mill Road. During this time the two 24-inch diameter pipes remain open. It is possible to release additional amounts of water when necessary through a series of gates.

Revised Analyses for the February 9, 2000, Countywide FIS

Water-surface elevations of floods of the selected recurrence intervals were computed using the USACE HEC-2 step-backwater computer program (Reference 36). Starting water-surface elevations were calculated using the slope/area method.

Channel and overbank roughness factors (Manning's 'n') used in the revised hydraulic computations were chosen based on field observations, experience with similar streams, and engineering judgment.

Revised Analyses for the February 20, 2002, Countywide FIS

Cross-sections were obtained by field surveys and information obtained from topographic maps at a scale of 1"=200' with a contour interval of 5 feet (Reference 34). Bridges, dams, and culverts were field checked to obtain elevation data and structural geometry.

Channel and overbank roughness factors (Manning's 'n') used in the revised hydraulic computations were chosen based on field observations, experience with similar streams, and engineering judgment.

It was determined that the levee along the east bank of the Congaree River would affect the flood hazard potential of this area of the county. Therefore, two analyses were computed for this stretch of the Congaree River, one with the levee and one without the levee. The first analysis represents a 100-year elevation on the waterward side of the levee should the levee remain intact. The second analysis represents flood conditions should the levee fail to provide protection against a 100-year event.

The floodway was computed assuming that the levee fails. The data for the Congaree River in the "Floodway Data Table" shows regulatory elevations for both the with and without levee scenarios. However, the "*With Floodway*" and "*Without Floodway*" elevations are based solely on the without levee scenario for the entire length of the Congaree River.

The topography of the Congaree River channel and the left overbank changes significantly approximately one mile downstream of the City of Columbia. The Congaree River channel becomes shallower with its flood conveyance considerably reduced compared to the channel upstream. The left overbank floodplain is relatively flat without high grounds to contain the floodwaters of the Congaree River. The technique used to approximate this flow situation was to assume that the effective one-dimensional overbank flow exists only along a portion of the floodplain available on the left overbank. Flow expansions have been observed to happen at angles of 14 to 20 degrees from the main direction of flow. Effective flow areas in the vicinity of flow expansions, and in the vicinity of the Interstate Route 77 road bridge, were defined using two-dimensional flow analyses and this assumption.

The channel roughness factors for the Congaree River ranged from 0.028 to 0.095 for the channel and from 0.030 to 0.200 for the overbanks.

For all streams studied by detailed methods prior to the current revision the channel and overbank roughness coefficients (Manning's 'n') are compiled in Table 13.

Table 13 – Summary of Roughness Coefficients (Historical)

Flooding Source	Manning's 'n' Channel	Manning's 'n' Overbank
Big Branch	0.100	0.100–0.120
Bear Creek	0.035–0.046	0.110–0.150
Congaree Creek	0.028–0.045	0.088–0.160

Table 13 – Summary of Roughness Coefficients (Historical) – continued

Flooding Source	Manning's 'n' Channel	Manning's 'n' Overbank
Congaree River	0.028–0.095	0.030–0.200
Fourteen Mile Creek	0.044–0.047	0.110–0.160
Hunt Branch	0.035–0.046	0.110–0.150
Koon Branch	0.040–0.120	0.040–0.150
Lick Fork Branch	0.035–0.048	0.110–0.160
Rawls Creek	0.040–0.120	0.040–0.150
Red Bank Creek	0.035–0.049	0.110–0.160
Saluda River	0.040–0.120	0.040–0.150
Savana Branch	0.035–0.055	0.110–0.150
Second Creek	0.035–0.046	0.110–0.150
Senn Branch	0.030–0.120	0.040–0.150
Six Mile Creek	0.040–0.080	0.040–0.150
Stoop Creek	0.025–0.100	0.060–0.180
Tributary CR-1	0.040–0.120	0.040–0.150
Tributary CR-1-1	0.040–0.120	0.040–0.150
Tributary K-2	0.025–0.100	0.060–0.180
Tributary R-2	0.040–0.120	0.040–0.150
Tributary SM-2	0.025–0.100	0.060–0.180
Tributary SM-3	0.030–0.120	0.040–0.150
Tributary SM-5	0.050–0.110	0.050–0.110
Tributary to Fourteen Mile Creek	0.060–0.080	0.080–0.120
Twelve Mile Creek	0.035–0.048	0.100–0.160
Yost Creek	0.025–0.100	0.060–0.180

This Countywide Analysis

For this report, streams which were studied were divided into four classifications; approximate, enhanced approximate, limited detailed, and detailed, based on their method of study. For approximate streams, a total of 222.9 miles were studied. For limited detailed streams, a total of 14.9 miles and 42 hydraulic structures were studied. For detailed streams, a total of 5.0 miles

and 17 hydraulic structures were studied. Hydraulic structures are defined as bridges, culverts, or dams.

Hydraulic cross-section geometries were obtained from LiDAR data. Hydraulic structures were field surveyed to obtain elevation data and structural geometry.

Water-surface elevations (WSELs) along each stream segment for the 10-, 4-, 2-, 1-, and 0.2-percent-annual-chance exceedance discharges for both limited detailed and detailed methods were computed using the USACE Hydrologic Engineering Center – River Analysis System (HEC-RAS) version 4.1 step-backwater computer program (Reference 37). WSELs were produced for the 1-percent-annual-chance exceedance discharges for approximate and enhanced approximate studies.

If applicable, a tie-in water-surface elevation was used as the starting condition for various hydraulic models. Otherwise, model starting conditions were set to normal depth using starting slopes calculated from channel elevation values taken from the LiDAR data.

Manning's n-values were estimated using USGS Digital Orthophoto Quarter Quads (DOQQ) for both channel and overbank areas. Manning's n-values for streams studied by methods other than detailed, ranged from 0.040 to 0.052 for the channel and from 0.040 to 0.150 for the overbanks. Roughness coefficients used for streams studied by detailed methods are given in Table 14.

Table 14 – Summary of Roughness Coefficients (Current Revision)

Flooding Source	Manning's 'n' Channel	Manning's 'n' Overbank
Broad River	0.040-0.050	0.040–0.150
Congaree River	0.035-0.038	0.020–0.125
Kinley Creek	0.049–0.055	0.060–0.150
Kinley Creek Tributary 1	0.050–0.052	0.100–0.150
Kinley Creek Tributary 2	0.050–0.053	0.060–0.150
Rawls Creek	0.048	0.060–0.150
Rawls Creek Tributary 2	0.048	0.060–0.110
Saluda River	0.040–0.050	0.040–0.150
Stoop Creek	0.045–0.055	0.060–0.160

The Congaree River hydraulic modeling was guided by the Levee Analysis and Mapping Procedure (LAMP) guidance. According to the LAMP process, a

Local Levee Partnership Team (LLPT) was established to guide the modeling process (Reference 39). Water-surface elevations of floods of the selected recurrence intervals were computed using a 1-dimensional Unsteady HEC-RAS model. Starting water-surface elevations were calculated using the slope/area method.

It was determined that the levee along the east bank of the Congaree does affect the flood hazard potential in this area of the county. The LAMP process required an existing conditions analysis as well as a natural valley analysis in the levee region.

The floodway was computed using the existing conditions analysis. The floodway data table for this area shows regulatory elevations for both scenarios mentioned above.

The topography of the Congaree River channel and overbanks changes significantly approximately one mile downstream of the City of Columbia. The Congaree River channel becomes shallower with its flood conveyance considerably reduced compared to the upstream channel. The overbank floodplains are relatively flat without significant high ground to contain the floodwaters of the River. A 2-dimensional FLO-2D model was incorporated to assist in establishing overland flow, potential storage areas as well as ineffective areas. The information from the 2-D model was used to verify or establish assumptions made in the 1-D HEC-RAS model.

3.3 Vertical Datum

All FIS reports and FIRM panels are referenced to a specific vertical datum. The vertical datum provides a starting point against which flood, ground, and structure elevations can be referenced and compared. Until recently, the standard vertical datum in use for newly created or revised FIS reports and FIRM panels was the National Geodetic Vertical Datum of 1929 (NGVD29). With the finalization of the North American Vertical Datum of 1988 (NAVD88), many FIS reports and FIRM panels are being prepared using NAVD88 as the referenced vertical datum.

All flood elevations shown in this FIS report and on the FIRM are referenced to the NAVD88. Structure and ground elevations in the community must, therefore, be referenced to NAVD88. In this current revision redelineated elevations from prior FIS reports were subjected to a vertical datum shift of -0.750 feet from NGVD29 to NAVD88 for Lexington County. It is important to note that adjacent communities may be referenced to NGVD29. This may result in differences in base flood elevations (BFEs) across the corporate limits between the communities.

For more information on NAVD88, see Converting the National Flood

Insurance Program to the North American Vertical Datum of 1988, FEMA Publication FIA-20/June 1992, or contact the Vertical Network Branch, National Geodetic Survey, Coast and Geodetic Survey, National Oceanic and Atmospheric Administration, Rockville, Maryland 20910 (Internet address http://www.ngs.noaa.gov).

Temporary vertical monuments are often established during the preparation of a flood hazard analysis for the purpose of establishing local vertical control. Although these monuments are not shown on the FIRM, they may be found in the archived project documentation associated with the FIS report and the FIRM panels for this community. Interested individuals may contact FEMA to access these data.

To obtain current elevation, description, and/or location information for benchmarks in the area, please contact the Information Services Branch of the NGS at (301) 713-3242, or visit their website at www.ngs.noaa.gov.

4.0 FLOODPLAIN MANAGEMENT APPLICATIONS

The NFIP encourages State and local governments to adopt sound floodplain management programs. To assist in this endeavor, each FIS provides 1-percent-annual-chance floodplain data, which may include a combination of the following: 10-, 2-, 1-, and 0.2-percent-annual-chance flood elevations; delineations of the 1- and 0.2-percent-annual-chance floodplains; and a 1-percent-annual-chance floodway. This information is presented on the FIRM and in many components of the FIS, including Flood Profiles, Floodway Data Tables, and Summary of Stillwater Elevation Tables. Users should reference the data presented in the FIS as well as additional information that may be available at the local community map repository before making flood elevation and/or floodplain boundary determinations.

4.1 Floodplain Boundaries

To provide a national standard without regional discrimination, the 1-percent-annual-chance flood has been adopted by FEMA as the base flood, also called the Special Flood Hazard Area (SFHA), for floodplain management purposes. The 0.2-percent-annual-chance flood is employed to indicate additional areas of flood risk in the community.

The 1- and 0.2-percent-annual-chance floodplain boundaries are shown on the FIRM. On this map, the 1-percent-annual-chance floodplain boundaries correspond to the boundaries of the areas of special food hazard (Zones A, and AE), and the 0.2-percent-annual-chance floodplain boundaries correspond to the boundaries of areas of moderate flood hazard. In cases where the 1- and 0.2-percent-annual-chance floodplain boundaries are close together, only the 1-percent-annual-chance floodplain boundaries have been shown. Small areas within the floodplain boundaries may lie above the flood elevations but cannot

be shown due to limitations of the map scale and/or lack of detailed topographic data.

In this countywide FIS the streams studied by approximate methods have only the 1-percent-annual-chance floodplain boundaries delineated on the FIRM. Those stream studied by limited detailed or detailed methods have the 1- and 0.2-percent-annual-chance floodplain boundaries shown on the FIRM. The boundaries were interpolated from flood elevations determined at each cross-section using LiDAR data at a scale of 1 meter with contour interval of 1 foot (Reference 38).

The boundaries of streams that were previously studied by approximate, limited detailed, and detailed methods and have not been restudied in this revision have been redelineated using more up-to-date topographic information.

4.2 Floodways

Encroachment on floodplains, such as structures and fill, reduces flood-carrying capacity, increases flood heights and velocities, and increases flood hazards in areas beyond the encroachment itself. One aspect of floodplain management involves balancing the economic gain from floodplain development against the resulting increase in flood hazard. For purposes of the National Flood Insurance Program, a floodway is used as a tool to assist local communities in this aspect of floodplain management. Under this concept, the area of the 1-percent-annual-chance floodplain is divided into a floodway and a floodway fringe. The floodway is the channel of a stream, plus any adjacent floodplain areas, that must be kept free of encroachment so that the 1-percent-annual-chance flood can be carried without substantial increases in flood heights. Minimum Federal standards limit such increases to 1.0 foot, provided that hazardous velocities are not produced. The floodways in this study are presented to local agencies as a minimum standard that can be adopted directly or that can be used as a basis for additional floodway studies.

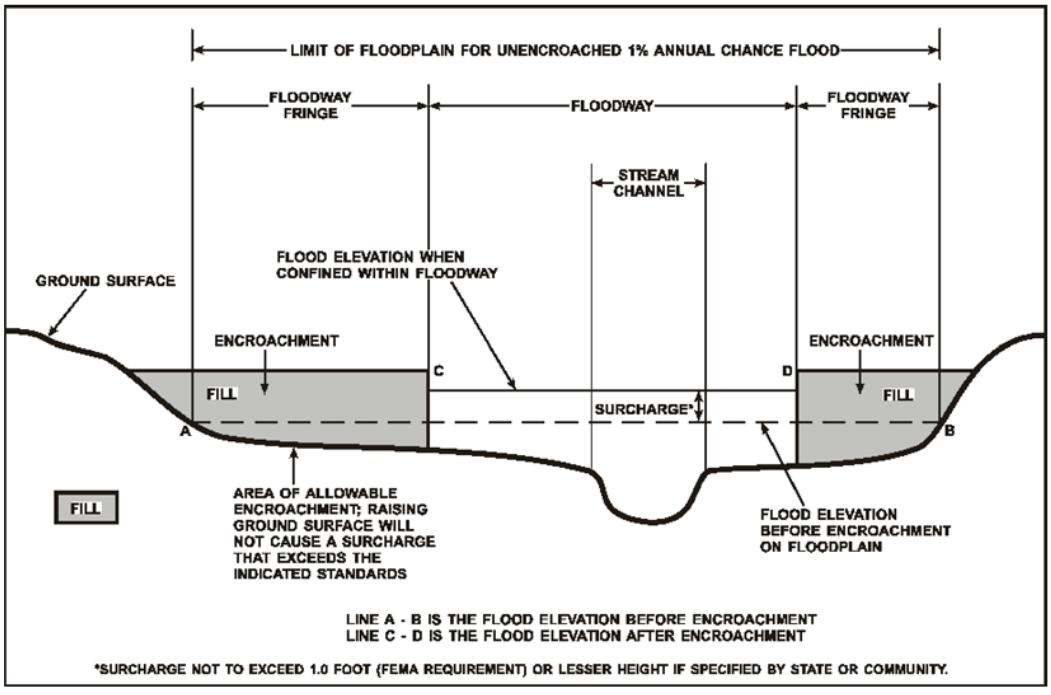
The floodways presented in this study were computed for certain stream segments on the basis of equal-conveyance reduction from each side of the floodplain. Floodway widths were computed at cross-sections. Between cross-sections, the floodway boundaries were interpolated. The results of the floodway computations for detailed studied stream are tabulated for selected cross-sections in Table 15, “Floodway Data”. The computed floodway is shown on the FIRM. In cases where the floodway and 1-percent-annual-chance floodplain boundaries are either too close together or collinear, only the floodway boundary is shown. Similarly, for limited detailed studied streams, BFE computations have been compiled in Table 16, “Flood Hazard Data for Selected Streams”.

Near the confluences of streams studied in detail, floodway computations were made without regard to flood elevations on the receiving water body. Therefore, "*Without Floodway*" elevations presented in Table 15, for certain downstream cross-sections of selected streams are lower than the regulatory flood elevations in that area, which must take into account the 1-percent-annual-chance flooding due to backwater from other sources.

Encroachment into areas subject to inundation by floodwaters having hazardous velocities aggravates the risk of flood damage, and heightens potential flood hazards by further increasing velocities. A listing of stream velocities at selected cross-sections is provided in Table 15. In order to reduce the risk of property damage in areas where the stream velocities are high, the community may wish to restrict development in areas outside the floodway.

The area between the floodway and 1-percent-annual-chance floodplain boundaries is termed the floodway fringe. The floodway fringe encompasses the portion of the floodplain that could be completely obstructed without increasing the water-surface elevation of the 1-percent-annual-chance flood by more than 1.0 foot at any point. Typical relationships between the floodway and the floodway fringe and their significance to floodplain development are shown in Figure 1.

Figure 1 – Floodway Schematic



LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	470	304	6,928	0.3	431.7	431.7	432.6	0.9
B	2,000	44	362	3.6	431.7	431.7	432.6	0.9
C	2,190	45	124	11.3	433.6	433.6	434.0	0.4

[1] Feet above Beechwoods Drive

TABLE 15

FEDERAL EMERGENCY MANAGEMENT AGENCY

LEXINGTON COUNTY, SOUTH CAROLINA

AND INCORPORATED AREAS

FLOODWAY DATA

BEACH CREEK

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	680	33	145	2.6	431.7	431.7	432.6	0.9
B	1,240	32	49	9.7	441.9	441.9	442.0	0.1
C	1,980	34	146	3.3	465.1	465.1	465.8	0.7

[1] Feet above confluence with Beach Creek

TABLE 15

FEDERAL EMERGENCY MANAGEMENT AGENCY

LEXINGTON COUNTY, SOUTH CAROLINA

AND INCORPORATED AREAS

FLOODWAY DATA

BEACH CREEK TRIBUTARY

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	4,500	110	667	1.2	182.6	182.6	183.6	1.0
B	5,000	215	1,267	0.7	182.9	182.9	183.9	1.0
C	6,100	100	374	2.0	188.7	188.7	189.7	1.0
D	7,700	130	551	1.1	190.7	190.7	191.7	1.0

[1] Feet above confluence with Congaree Creek

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH[2] (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	0	326 / 1,512	40,919	5.9	153.9	153.9	154.3	0.4
B	1,000	465 / 1,950[3]	36,058	6.7	154.5	154.5	154.7	0.2

[1] Feet above confluence with Congaree River
[2] Width within county / total width
[3] Combined floodway width for Broad River / Saluda River

TABLE 15

FEDERAL EMERGENCY MANAGEMENT AGENCY

LEXINGTON COUNTY, SOUTH CAROLINA

AND INCORPORATED AREAS

FLOODWAY DATA

BROAD RIVER

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	24,780	1,020	4,984	1.9	140.9	124.9[3]	125.9	1.0
B	39,800	320	2,192	4.3	140.9	132.3[3]	133.1	0.8
C	40,600	80	2,487	3.8	140.9	133.5[3]	134.4	0.9
D	41,600	400	1,973	4.8	140.9	139.1[3]	139.8	0.7
E	44,600	780	7,034	1.3	142.1	142.1	142.7	0.6
F	51,200	800	5,747	1.7	142.3	142.3	143.1	0.8
G	53,400	805	4,758	2.0	142.6	142.6	143.6	1.0
H	55,500	880[2]	4,112	2.2	143.6	143.6	144.6	1.0
I	61,900	443	1,160	7.9	147.5	147.5	147.6	0.1
J	62,250	443	1,347	6.8	148.0	148.0	148.0	0.0
K	68,000	178	1,027	6.8	155.0	155.0	156.0	1.0
L	69,100	90	585	11.9	156.7	156.7	156.7	0.0
M	73,500	530	3,087	2.2	160.0	160.0	160.7	0.7
N	78,100	640	2,112	3.3	162.6	162.6	163.6	1.0
O	79,791	450	2,101	1.9	164.8	164.8	165.8	1.0
P	80,907	225	1,146	3.5	167.1	167.1	167.6	0.5
Q	81,453	149	844	4.8	168.3	168.3	169.1	0.8
R	82,463	342	1,754	2.3	170.8	170.8	171.7	0.9
S	83,114	203	961	4.2	172.0	172.0	172.7	0.7
T	85,802	378	4,302	1.0	184.7	184.7	185.5	0.8
U	86,624	180	1,893	2.2	184.7	184.7	185.5	0.8
V	87,560	250	1,699	2.5	184.7	184.7	185.4	0.7
W	89,271	213	1,236	3.4	187.5	187.5	188.4	0.9
X	90,217	579	3,845	1.1	188.8	188.8	189.8	1.0

[1] Feet above confluence with Congaree River
[2] Combined Congaree Creek / Savana Branch floodway
[3] Elevation computed without consideration of backwater effects from Congaree River

TABLE 15

FEDERAL EMERGENCY MANAGEMENT AGENCY

LEXINGTON COUNTY, SOUTH CAROLINA

AND INCORPORATED AREAS

FLOODWAY DATA

CONGAREE CREEK

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Y	90,773	572	3,840	1.1	189.1	189.1	190.1	1.0
Z	91,840	181	951	4.4	189.6	189.6	190.3	0.7
AA	92,776	579	3,426	1.2	191.5	191.5	192.5	1.0
AB	94,500	345	1,831	2.3	193.8	193.8	194.8	1.0
AC	95,393	347	1,328	3.1	195.9	195.9	196.7	0.8
AD	96,381	603	2,778	1.5	198.2	198.2	199.2	1.0
AE	97,404	162	1,083	3.9	199.2	199.2	200.1	0.9
AF	98,378	115	858	4.9	200.2	200.2	201.1	0.9
AG	99,447	270	1,668	2.5	201.2	201.2	202.2	1.0
AH	101,133	130	844	4.8	205.0	205.0	205.2	0.2
AI	101,906	110	820	4.9	205.6	205.6	206.0	0.4
AJ	102,981	120	667	6.1	206.7	206.7	207.5	0.8
AK	103,880	170	846	4.8	208.9	208.9	209.8	0.9
AL	104,805	240	1,053	3.9	211.1	211.1	211.9	0.8
AM	105,810	360	2,332	1.7	212.6	212.6	213.5	0.9
AN	106,521	420	1,691	2.4	213.2	213.2	214.2	1.0
AO	108,261	350	1,604	2.5	219.8	219.8	220.5	0.7
AP	109,077	400	2,397	1.7	220.8	220.8	221.8	1.0
AQ	109,871	400	2,237	1.8	221.8	221.8	222.5	0.7
AR	110,876	390	1,972	2.1	222.9	222.9	223.6	0.7
AS	111,719	220	953	3.3	224.0	224.0	225.0	1.0
AT	112,629	160	695	4.5	227.5	227.5	228.1	0.6
AU	113,470	135	773	4.0	230.1	230.1	231.1	1.0
AV	114,359	180	678	4.6	233.2	233.2	233.7	0.5

[1] Feet above confluence with Congaree River

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
AW	115,387	170	767	4.1	238.2	238.2	238.9	0.7
AX	115,965	140	749	4.2	240.2	240.2	240.9	0.7
AY	118,643	968	5,891	0.5	255.2	255.2	255.2	0.0
AZ	119,852	544	2,307	1.2	255.2	255.2	255.2	0.0
BA	120,512	383	1,260	2.2	255.3	255.3	255.3	0.0
BB	121,284	350	1,246	2.3	256.1	256.1	256.1	0.0
BC	122,156	240	1,284	2.2	258.0	258.0	259.0	1.0
BD	123,030	200	1,051	2.7	260.5	260.5	261.5	1.0
BE	123,854	140	769	3.7	263.8	263.8	264.6	0.8
BF	124,886	270	1,022	2.8	268.2	268.2	269.2	1.0
BG	125,779	130	736	3.8	272.1	272.1	272.8	0.7
BH	126,740	260	1,651	1.7	274.4	274.4	275.4	1.0
BI	127,616	132	760	3.7	276.4	276.4	277.1	0.7
BJ	128,423	380	2,092	1.4	278.4	278.4	279.3	0.9

[1] Feet above confluence with Congaree River

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH[2] (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	232,472	347 / 8,654	102,549	2.8	129.0 / 128.0[3]	129.0	129.8	0.8
B	242,472	5,740 / 6,312	87,267	3.3	131.9 / 130.5[3]	131.9	132.8	0.9
C	246,078	3,538 / 5,733	77,518	3.7	133.8 / 131.8[3]	133.8	134.6	0.8
D	254,766	7,243 / 8,277	137,379	2.1	136.8 / 134.4[3]	136.8	137.6	0.8
E	257,368	6,436 / 6,872	121,107	2.4	137.9 / 135.6[3]	137.9	138.6	0.7
F	260,635	5,446 / 5,873	87,103	3.3	139.0 / 139.0[3]	139.0	139.7	0.7
G	263,569	4,543 / 4,995	76,750	3.7	139.8 / 139.7[3]	139.8	140.5	0.7
H	266,472	3,583 / 3,949	63,695	4.5	140.7 / 140.4[3]	140.7	141.3	0.6
I	269,529	560 / 896	27,167	10.5	141.6 / 141.3[3]	141.6	142.3	0.7
J	272,318	371 / 813	25,013	11.4	142.7 / 142.2[3]	142.7	143.5	0.8
K	275,472	627 / 880	28,647	10.0	144.9 / 144.5[3]	144.9	145.9	1.0
L	278,919	1,098 / 1,811	49,779	5.7	149.0 / 148.8[3]	149.0	149.7	0.7
M	283,052	571 / 1,785	47,724	6.0	150.6 / 150.4[3]	150.6	151.2	0.6
N	287,472	944 / 2,733	54,370	5.3	153.9 / 153.6[3]	153.9	154.3	0.4

[1] Feet above confluence with Wateree River

[2] Width within county / total width

[3] Elevations computed With Levee Effects / Natural Valley Effects

TABLE 15

FEDERAL EMERGENCY MANAGEMENT AGENCY

LEXINGTON COUNTY, SOUTH CAROLINA

AND INCORPORATED AREAS

FLOODWAY DATA

CONGAREE RIVER

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	6,850	151	1,980	0.6	164.9	164.9	164.9	0.0
B	7,150	150	1,566	0.8	164.9	164.9	164.9	0.0
C	7,600	160	1,272	1.0	165.0	165.0	165.1	0.1
D	8,200	130	571	2.2	165.2	165.2	166.0	0.8
E	9,350	44	134	9.2	176.3	176.3	176.7	0.4
F	10,725	181	982	1.3	185.7	185.7	186.4	0.7
G	11,050	300	3,695	0.3	196.5	196.5	196.5	0.0
H	11,530	193	1,420	0.9	196.6	196.6	196.6	0.0
I	11,850	147	496	2.5	196.7	196.7	196.7	0.0
J	12,650	199	956	1.3	197.9	197.9	198.2	0.3
K	13,650	47	138	8.9	203.2	203.2	203.7	0.5
L	14,300	97	481	2.6	212.6	212.6	213.4	0.8
M	15,250	45	166	7.4	218.9	218.9	219.7	0.8

[1] Feet above confluence with Congaree River

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	600	50	106	9.1	202.1	202.1	203.1	1.0
B	850	100	781	1.2	213.1	213.1	214.1	1.0
C	1,180	215	2,785	0.3	225.7	225.7	226.7	1.0
D	1,580	90	700	1.4	226.0	226.0	226.9	0.9
E	2,200	60	165	5.8	227.4	227.4	227.9	0.5
F	2,860	100	698	1.4	235.7	235.7	236.7	1.0

[1] Feet above confluence with Congaree River Tributary 1

TABLE 15

FEDERAL EMERGENCY MANAGEMENT AGENCY

LEXINGTON COUNTY, SOUTH CAROLINA

AND INCORPORATED AREAS

FLOODWAY DATA

CONGAREE RIVER TRIBUTARY 1-1

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	1,727	692	3,086	1.9	155.7	155.7	156.1	0.4
B	2,372	591	3,131	1.9	157.0	157.0	157.8	0.8
C	4,368	650	2,599	2.3	163.3	163.3	164.3	1.0
D	4,957	630	4,430	1.3	164.4	164.4	165.1	0.7
E	5,639	498	2,555	2.3	164.8	164.8	165.5	0.7
F	6,621	478	2,386	2.6	167.0	167.0	167.3	0.3
G	7,371	408	2,488	2.5	168.7	168.7	169.2	0.5
H	8,718	590	4,641	1.3	172.9	172.9	173.8	0.9
I	9,394	180	1,338	4.6	173.2	173.2	174.2	1.0
J	10,355	292	2,626	2.3	175.3	175.3	176.3	1.0
K	11,594	150	1,131	5.3	177.9	177.9	178.7	0.8
L	12,591	242	1,492	2.5	181.6	181.6	182.6	1.0
M	13,576	250	1,254	3.0	184.9	184.9	185.7	0.8
N	14,411	400	1,705	2.2	188.5	188.5	189.2	0.7
O	15,559	469	1,230	3.1	195.4	195.4	195.5	0.1
P	16,134	427	2,114	1.8	197.5	197.5	198.2	0.7
Q	17,139	370	1,247	3.0	200.8	200.8	201.7	0.9
R	17,482	270	1,386	2.7	202.9	202.9	203.9	1.0
S	18,373	105	523	7.1	208.4	208.4	209.0	0.6
T	19,122	128	1,028	3.6	213.5	213.5	214.4	0.9
U	20,211	250	2,356	1.6	223.3	223.3	224.1	0.8
V	20,831	330	2,953	1.3	223.3	223.3	224.1	0.8
W	21,464	280	1,948	1.9	223.3	223.3	224.1	0.8
X	22,251	240	1,449	2.6	224.6	224.6	225.2	0.6

[1] Feet above confluence with Congaree Creek

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Y	22,951	300	1,380	2.7	226.6	226.6	227.6	1.0
Z	23,497	330	2,198	1.7	228.2	228.2	229.1	0.9
AA	24,323	265	1,627	1.9	229.3	229.3	230.3	1.0
AB	25,217	155	1,080	2.9	231.2	231.2	232.0	0.8
AC	25,812	150	778	4.0	233.3	233.3	234.3	1.0
AD	26,627	255	1,360	2.3	237.3	237.3	238.3	1.0
AE	27,562	235	1,340	2.3	239.9	239.9	240.9	1.0
AF	28,387	185	655	4.5	244.2	244.2	244.9	0.7
AG	28,895	280	1,306	2.2	247.9	247.9	248.9	1.0
AH	30,559	401	2,907	1.0	259.5	259.5	259.7	0.2
AI	31,372	200	1,194	2.5	260.0	260.0	260.3	0.3
AJ	32,187	305	1,682	1.7	261.8	261.8	262.7	0.9
AK	33,003	255	1,217	2.4	264.2	264.2	265.2	1.0
AL	33,568	96	586	5.0	267.5	267.5	268.2	0.7
AM	34,112	210	1,375	2.1	270.3	270.3	271.3	1.0
AN	34,882	385	2,335	1.3	271.6	271.6	272.6	1.0
AO	35,636	190	613	4.8	273.7	273.7	274.4	0.7
AP	36,203	223	1,240	2.4	278.4	278.4	279.4	1.0
AQ	36,970	277	1,368	0.9	280.0	280.0	281.0	1.0
AR	37,643	80	231	5.5	283.4	283.4	284.0	0.6
AS	38,343	100	564	2.1	292.5	292.5	293.5	1.0

[1] Feet above confluence with Congaree Creek

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	2,535	220	1,282	4.6	192.0	192.0	193.0	1.0
B	4,765	457	2,235	2.6	203.9	203.9	204.9	1.0
C	8,535	402	1,652	3.5	223.6	223.6	224.5	0.9
D	10,300	150	1,275	4.5	232.5	232.5	233.3	0.8
E	10,845	200	1,638	3.5	234.1	234.1	235.0	0.9
F	13,580	184	1,430	4.0	240.4	240.4	241.4	1.0
G	15,310	89	778	7.3	248.9	248.9	249.8	0.9
H	15,490	111	1,000	5.7	252.5	252.5	252.9	0.4
I	17,545	132	1,454	3.9	262.5	262.5	263.5	1.0
J	18,660	304	2,456	2.3	264.4	264.4	265.4	1.0
K	19,190	270	2,806	2.0	264.8	264.8	265.8	1.0
L	19,300	138	1,122	5.0	264.8	264.8	265.6	0.8
M	19,455	116	939	6.0	264.8	264.8	265.8	1.0
N	19,770	124	983	5.8	265.7	265.7	266.6	0.9
O	20,035	162	1,343	4.2	266.5	266.5	267.3	0.8
P	20,140	100	717	7.9	266.5	266.5	267.1	0.6
Q	20,345	124	1,143	5.0	268.9	268.9	269.4	0.5
R	20,570	161	974	5.8	269.1	269.1	269.6	0.5
S	20,640	201	1,374	4.1	269.4	269.4	270.2	0.8
T	21,000	88	677	8.4	271.0	271.0	271.5	0.5
U	21,135	70	504	10.6	272.1	272.1	272.9	0.8
V	21,200	79	671	8.0	273.7	273.7	274.5	0.8
W	21,250	115	987	5.4	274.5	274.5	275.4	0.9
X	21,645	171	1,081	4.9	276.2	276.2	277.1	0.9

[1] Feet above confluence with Twelve Mile Creek

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Y	22,000	115	773	6.9	278.2	278.2	279.0	0.8
Z	22,165	131	954	5.6	279.5	279.5	280.4	0.9
AA	23,640	214	1,662	3.2	285.9	285.9	286.9	1.0
AB	25,605	497	2,691	2.0	291.8	291.8	292.8	1.0
AC	25,820	486	3,662	1.5	294.8	294.8	295.1	0.3
AD	26,080	226	1,001	4.9	295.1	295.1	295.6	0.5
AE	26,260	310	2,208	2.2	297.6	297.6	298.5	0.9
AF	28,485	286	1,277	3.9	307.3	307.3	308.2	0.9
AG	30,475	249	1,492	3.2	320.2	320.2	320.9	0.7
AH	30,695	315	2,914	1.6	323.5	323.5	324.5	1.0
AI	32,410	365	1,412	3.4	327.4	327.4	328.4	1.0
AJ	32,680	310	2,039	2.3	330.3	330.3	330.8	0.5
AK	33,845	162	843	5.3	337.1	337.1	338.0	0.9
AL	34,110	240	2,781	1.6	346.0	346.0	347.0	1.0
AM	35,405	166	1,128	3.9	347.1	347.1	348.0	0.9
AN	37,280	195	841	2.7	356.8	356.8	357.8	1.0
AO	38,241	195	1,030	2.2	360.7	360.7	361.1	0.4
AP	38,993	185	645	3.5	363.3	363.3	364.3	1.0
AQ	39,670	132	644	3.5	367.4	367.4	368.2	0.8
AR	40,493	100	400	1.1	369.7	369.7	370.7	1.0
AS	40,988	24	51	8.4	374.3	374.3	374.8	0.5
AT	41,628	262	2,170	0.2	392.8	392.8	393.6	0.8
AU	43,140	41	61	7.0	399.0	399.0	399.0	0.0
AV	43,851	41	101	4.3	407.0	407.0	407.1	0.1

[1] Feet above confluence with Twelve Mile Creek

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
AW	44,889	60	128	3.3	417.0	417.0	417.6	0.6
AX	45,579	100	320	1.3	430.3	430.3	431.2	0.9
AY	46,409	80	132	3.3	436.8	436.8	437.2	0.4
AZ	47,387	70	229	1.9	447.8	447.8	448.7	0.9
BA	47,737	65	125	3.4	451.0	451.0	451.6	0.6

[1] Feet above confluence with Twelve Mile Creek

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	5,000	460	2,484	2.4	191.2	183.1[2]	183.9	0.8
B	6,500	113	1,499	4.0	191.6	191.6	192.3	0.8
C	7,358	173	2,084	2.9	193.6	193.6	194.2	0.6
D	8,500	484	2,949	1.8	194.9	194.9	195.7	0.8
E	9,881	258	1,433	2.2	198.6	198.6	199.5	0.9
F	11,382	98	638	5.0	204.9	204.9	205.4	0.5
G	12,291	265	1,370	2.3	209.1	209.1	209.9	0.8
H	12,740	201	1,200	2.6	210.1	210.1	210.9	0.8
I	14,550	185	581	4.7	213.9	213.9	214.9	1.0
J	16,250	111	883	3.1	222.0	222.0	222.5	0.5
K	16,400	111	717	2.8	222.3	222.3	222.9	0.6
L	17,450	129	530	3.8	227.6	227.6	228.3	0.7
M	18,200	43	430	4.7	231.0	231.0	232.0	1.0

[1] Feet above confluence with Saluda River

[2] Elevation computed without consideration of backwater effects from Saluda River

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	500	54	145	6.3	194.1	190.3[2]	190.7	0.4
B	1,000	65	290	3.2	197.2	197.2	198.1	0.9
C	2,000	45	152	6.1	206.8	206.8	207.1	0.3
D	3,031	39	140	6.6	218.0	218.0	218.4	0.4
E	3,756	40	172	5.4	226.1	226.1	226.6	0.5
F	4,520	88	264	3.5	238.0	238.0	238.2	0.1
G	5,153	43	161	5.7	246.0	246.0	246.7	0.7
H	5,928	146	246	3.8	255.8	255.8	256.6	0.8
I	6,432	50	201	4.6	266.8	266.8	267.3	0.5

[1] Feet above confluence with Kinley Creek

[2] Elevation computed without consideration of backwater effects from Kinley Creek

TABLE 15

FEDERAL EMERGENCY MANAGEMENT AGENCY

LEXINGTON COUNTY, SOUTH CAROLINA

AND INCORPORATED AREAS

FLOODWAY DATA

KINLEY CREEK TRIBUTARY 1

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	213	273	1,039	2.5	197.5	197.5	198.2	0.7
B	1,000	147	770	3.3	202.0	202.0	202.9	0.9
C	2,000	148	722	3.5	209.9	209.9	210.1	0.2
D	2,888	177	784	3.2	214.8	214.8	215.7	0.9
E	3,804	180	718	3.5	220.3	220.3	221.0	0.7
F	4,500	200	1,256	1.8	228.0	228.0	228.3	0.3
G	5,459	207	905	2.5	231.2	231.2	232.2	1.0
H	7,147	40	221	10.0	245.6	245.6	246.4	0.8

[1] Feet above confluence with Kinley Creek

TABLE 15

FEDERAL EMERGENCY MANAGEMENT AGENCY

LEXINGTON COUNTY, SOUTH CAROLINA

AND INCORPORATED AREAS

FLOODWAY DATA

KINLEY CREEK TRIBUTARY 2

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	300	165	270	6.8	196.9	188.8[2]	189.8	1.0
B	800	200	534	3.4	196.9	192.2[2]	193.2	1.0
C	1,530	175	728	2.5	197.3	197.3	198.3	1.0
D	2,140	100	362	5.0	201.9	201.9	202.6	0.7
E	2,510	115	476	3.8	205.8	205.8	206.7	0.9
F	3,090	155	437	4.2	210.7	210.7	211.7	1.0
G	3,670	95	293	6.2	214.4	214.4	215.4	1.0
H	3,790	100	385	4.7	215.7	215.7	216.7	1.0
I	4,790	90	395	4.6	224.1	224.1	225.0	0.9
J	4,960	120	496	3.0	226.7	226.7	227.0	0.3
K	5,415	115	295	5.1	228.9	228.9	229.7	0.8
L	6,250	105	333	4.5	238.5	238.5	239.4	0.9
M	6,420	105	1,916	0.8	247.0	247.0	248.0	1.0
N	6,800	100	496	3.0	247.0	247.0	247.9	0.9
O	8,050	90	169	8.9	257.8	257.8	258.0	0.2
P	8,250	120	503	2.1	261.6	261.6	262.6	1.0
Q	8,750	120	252	4.1	263.8	263.8	264.3	0.5
R	8,900	65	158	6.6	267.1	267.1	268.1	1.0
S	9,250	50	232	4.5	271.6	271.6	272.6	1.0
T	9,350	50	230	4.5	272.3	272.3	273.2	0.9
U	9,570	50	320	3.2	273.9	273.9	274.9	1.0

[1] Feet above confluence with Rawls Creek

[2] Elevation computed without consideration of backwater effects from Saluda River

TABLE 15

FEDERAL EMERGENCY MANAGEMENT AGENCY

LEXINGTON COUNTY, SOUTH CAROLINA

AND INCORPORATED AREAS

FLOODWAY DATA

KOON BRANCH

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	178	260	656	2.5	184.1	184.0[2]	184.0	0.0
B	1,151	240	1,691	1.0	192.2	192.2	193.1	0.9
C	2,181	120	614	2.6	195.0	195.0	196.0	1.0
D	2,903	80	331	4.9	198.1	198.1	199.1	1.0
E	3,509	90	440	3.7	201.1	201.1	201.9	0.8
F	4,666	60	281	5.7	206.3	206.3	207.1	0.8
G	5,922	230	1,224	1.3	212.9	212.9	213.2	0.3
H	6,879	80	405	4.0	216.4	216.4	217.4	1.0
I	7,682	60	307	5.2	219.2	219.2	219.8	0.6
J	8,182	60	286	5.6	221.5	221.5	221.6	0.1
K	9,447	190	1,105	1.0	234.1	234.1	234.4	0.3
L	10,293	113	556	2.0	239.5	239.5	240.5	1.0
M	11,102	58	255	4.5	245.1	245.1	246.1	1.0
N	12,000	77	369	1.8	252.4	252.4	253.4	1.0
O	12,584	56	186	3.5	255.3	255.3	256.1	0.8

[1] Feet above confluence with Red Bank Creek

[2] Elevation computed without consideration of backwater effects from Red Bank Creek

TABLE 15

FEDERAL EMERGENCY MANAGEMENT AGENCY

LEXINGTON COUNTY, SOUTH CAROLINA

AND INCORPORATED AREAS

FLOODWAY DATA

LICK FORK BRANCH

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	1,510	100	292	2.2	385.9	385.9	385.9	0.0
B	1,672	220	823	5.1	387.4	387.4	388.2	0.8
C	2,011	274	1,367	2.0	394.8	394.8	394.9	0.1
D	2,321	180	810	1.2	395.3	395.3	395.3	0.0
E	2,551	135	318	2.0	397.0	397.0	397.4	0.4
F	2,752	155	741	5.6	399.2	399.2	400.1	0.9

[1] Feet above confluence with Fourteen Mile Creek

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	1,400	160	1,063	4.5	196.9	179.9[2]	180.9	1.0
B	2,600	170	1,004	4.8	196.9	185.1[2]	185.6	0.5
C	2,800	174	810	5.9	196.9	186.2[2]	186.3	0.1
D	4,100	130	1,369	3.1	196.9	192.7[2]	192.9	0.2
E	4,350	140	1,520	2.8	196.9	195.3[2]	195.4	0.1
F	5,000	102	1,380	3.1	196.9	195.5[2]	196.0	0.5
G	5,100	127	1,082	4.0	199.2	199.2	199.2	0.0
H	5,200	385	4,947	8.7	199.4	199.4	199.4	0.0
I	6,050	225	2,007	2.1	199.4	199.4	199.4	0.0
J	6,600	55	374	11.5	199.4	199.4	199.4	0.0
K	6,750	87	403	10.7	199.4	199.4	199.4	0.0
L	7,000	395	1,147	3.8	202.5	202.5	202.5	0.0
M	7,050	370	1,756	2.5	202.7	202.7	202.8	0.1
N	7,650	252	985	4.4	203.3	203.3	203.5	0.2
O	7,700	250	1,004	4.3	203.3	203.3	203.7	0.4
P	8,150	150	803	4.8	204.0	204.0	204.6	0.6
Q	9,400	100	540	7.1	208.2	208.2	208.7	0.5
R	10,150	47	353	10.9	214.6	214.6	215.2	0.6
S	10,250	90	738	5.2	216.5	216.5	217.0	0.5
T	11,000	55	429	8.9	217.8	217.8	218.6	0.8
U	12,100	102	533	7.2	225.9	225.9	226.4	0.5
V	13,300	64	529	7.3	232.0	232.0	233.0	1.0
W	13,850	172	1,218	3.2	236.4	236.4	237.4	1.0
X	14,750	114	796	4.8	240.5	240.5	241.2	0.7

[1] Feet above confluence with Saluda River

[2] Elevation computed without consideration of backwater effects from Saluda River

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Y	14,900	184	1,289	3.0	245.3	245.3	245.3	0.0
Z	15,800	66	744	5.2	250.2	250.2	250.7	0.5
AA	15,950	117	1,230	3.1	252.1	252.1	253.1	1.0
AB	16,900	241	1,564	2.5	254.1	254.1	254.9	0.8
AC	18,050	65	509	7.5	258.8	258.8	259.8	1.0
AD	18,200	155	2,092	1.6	260.0	260.0	260.9	0.9
AE	18,800	95	670	5.1	260.6	260.6	260.8	0.2
AF	19,500	85	403	8.5	267.5	267.5	268.5	1.0
AG	19,650	95	573	6.0	267.7	267.7	268.5	0.8
AH	20,900	110	518	6.6	270.7	270.7	271.1	0.4
AI	22,250	235	1,840	1.9	278.9	278.9	279.9	1.0
AJ	23,150	145	934	3.7	282.1	282.1	282.8	0.7
AK	23,300	170	1,879	1.8	285.3	285.3	285.3	0.0
AL	23,600	205	1,767	1.9	285.8	285.8	286.0	0.2
AM	25,500	200	948	2.9	292.4	292.4	293.4	1.0
AN	26,150	135	1,054	2.6	296.0	296.0	296.8	0.8
AO	27,480	96	368	5.5	302.5	302.5	303.4	0.9
AP	27,680	92	460	4.4	304.0	304.0	304.6	0.6
AQ	28,130	50	205	9.8	304.8	304.8	305.4	0.6
AR	28,320	27	151	13.3	307.2	307.2	307.2	0.0
AS	31,416	98	506	4.4	310.8	310.8	311.3	0.5

[1] Feet above confluence with Saluda River

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	1,365	64	250	7.6	299.6	299.6	300.5	0.9

[1] Feet above confluence with Rawls Creek

TABLE 15

FEDERAL EMERGENCY MANAGEMENT AGENCY

LEXINGTON COUNTY, SOUTH CAROLINA

AND INCORPORATED AREAS

FLOODWAY DATA

RAWLS CREEK TRIBUTARY 2

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	1,116	212	852	4.6	164.2	164.2	165.2	1.0
B	1,433	179	934	4.2	166.2	166.2	167.0	0.8
C	2,447	313	1,644	2.4	169.2	169.2	170.2	1.0
D	2,942	190	1,080	3.6	170.4	170.4	171.2	0.8
E	3,857	246	1,332	2.9	173.2	173.2	174.2	1.0
F	6,582	720	5,806	0.7	184.1	184.1	185.1	1.0
G	7,911	571	2,576	1.5	184.1	184.1	185.1	1.0
H	8,601	163	632	5.8	186.3	186.3	186.5	0.2
I	9,598	315	2,003	1.8	189.8	189.8	190.8	1.0
J	10,556	160	929	3.9	190.8	190.8	191.8	1.0
K	11,430	315	1,935	1.9	192.7	192.7	193.7	1.0
L	12,448	125	479	7.6	194.8	194.8	195.5	0.7
M	13,295	235	1,624	2.3	198.2	198.2	199.2	1.0
N	14,445	150	635	5.8	200.4	200.4	201.1	0.7
O	15,394	140	926	4.0	206.9	206.9	207.8	0.9
P	16,221	110	811	4.5	209.6	209.6	210.6	1.0
Q	17,207	230	1,088	3.4	214.2	214.2	214.8	0.6
R	18,198	140	628	5.8	220.0	220.0	220.8	0.8
S	19,128	320	2,098	1.7	223.9	223.9	224.9	1.0
T	19,810	286	1,652	2.2	225.0	225.0	225.9	0.9
U	21,132	294	1,695	2.0	228.8	228.8	229.8	1.0
V	22,155	190	863	4.0	230.4	230.4	231.2	0.8
W	23,080	70	465	7.4	235.1	235.1	236.0	0.9
X	26,435	639	6,840	0.5	260.1	260.1	261.1	1.0

[1] Feet above confluence with Congaree Creek

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Y	27,872	617	4,473	0.8	260.1	260.1	261.1	1.0
Z	29,111	270	1,392	2.5	260.1	260.1	261.1	1.0
AA	29,986	140	662	5.2	263.4	263.4	263.4	0.0
AB	31,780	93	832	4.1	271.0	271.0	271.6	0.6
AC	32,438	61	434	7.8	272.1	272.1	273.1	1.0
AD	33,529	326	2,708	1.3	273.5	273.5	274.4	0.9
AE	34,583	274	1,276	2.7	273.7	273.7	274.5	0.8
AF	35,533	96	490	6.9	280.7	280.7	280.8	0.1
AG	36,434	182	1,552	2.1	286.6	286.6	287.6	1.0
AH	37,443	120	969	3.4	289.4	289.4	290.2	0.8
AI	38,322	370	2,955	1.1	291.0	291.0	292.0	1.0
AJ	39,144	238	2,101	1.6	291.8	291.8	292.7	0.9
AK	40,075	183	1,352	2.4	293.2	293.2	294.1	0.9
AL	41,037	223	1,487	2.2	296.3	296.3	297.3	1.0
AM	42,787	245	1,749	1.9	299.6	299.6	300.6	1.0
AN	44,546	180	1,207	2.3	306.2	306.2	307.2	1.0
AO	45,507	194	1,230	2.3	308.5	308.5	309.3	0.8
AP	46,236	312	2,094	1.3	309.5	309.5	310.5	1.0
AQ	47,045	240	1,143	2.4	310.6	310.6	311.6	1.0
AR	47,858	215	921	3.0	314.9	314.9	315.3	0.4
AS	48,731	163	958	2.9	317.6	317.6	318.6	1.0
AT	49,538	210	921	3.0	320.6	320.6	321.6	1.0
AU	50,412	110	667	4.2	324.4	324.4	325.3	0.9
AV	51,662	155	1,846	1.5	335.8	335.8	336.2	0.4

[1] Feet above confluence with Congaree Creek

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
AW	52,542	180	1,234	2.3	335.9	335.9	336.7	0.8
AX	53,120	235	1,211	2.0	336.5	336.5	337.5	1.0
AY	54,043	110	465	5.1	339.0	339.0	339.5	0.5
AZ	54,991	90	569	4.2	343.0	343.0	343.7	0.7
BA	56,023	122	570	4.2	345.8	345.8	346.7	0.9
BB	56,744	95	329	7.3	348.9	348.9	349.0	0.1
BC	58,539	276	1,533	1.5	356.5	356.5	356.9	0.4
BD	59,416	150	406	5.7	359.4	359.4	359.6	0.2
BE	59,843	200	913	2.5	362.7	362.7	363.5	0.8
BF	61,685	80	558	1.8	370.1	370.1	370.8	0.7

[1] Feet above confluence with Congaree Creek

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	3,400	129 / 306[2]	4,638	12.6	154.7	146.3[3]	146.9	0.6
B	4,400	90 / 270[2]	4,474	13.1	154.7	149.6[3]	149.9	0.3
C	5,400	118 / 355[2]	6,281	9.3	154.7	152.9[3]	153.4	0.5
D	6,900	230 / 405[2]	7,796	7.5	155.0	155.0	155.4	0.4
E	7,900	602 / 845[2]	12,025	4.9	156.5	156.5	157.0	0.5
F	9,000	236 / 383[2]	6,177	9.5	157.1	157.1	157.5	0.4
G	10,000	98 / 385[2]	6,221	9.4	158.9	158.9	159.2	0.3
H	11,000	136 / 345[2]	5,556	10.6	160.7	160.7	161.0	0.3
I	12,000	101 / 273[2]	4,171	14.1	162.1	162.1	162.3	0.2
J	13,000	313 / 519[2]	7,028	8.3	166.2	166.2	166.2	0.0
K	14,200	578 / 740[2]	11,872	4.9	169.0	169.0	169.4	0.4
L	15,200	799 / 920[2]	9,676	6.1	169.4	169.4	169.9	0.5
M	16,200	477 / 710[2]	8,001	7.3	171.3	171.3	171.8	0.5
N	17,000	226 / 600[2]	8,255	7.1	177.6	177.6	177.9	0.3
O	18,800	1,428	23,432	4.5	180.9	180.9	181.5	0.6
P	22,000	939	15,390	6.8	183.3	183.3	184.0	0.7
Q	24,800	1,030	19,348	5.4	186.7	186.7	187.6	0.9
R	26,000	1,335	23,330	4.5	187.6	187.6	188.5	0.9
S	28,600	1,700	30,121	3.5	188.9	188.9	189.8	0.9
T	31,500	2,010	26,530	4.0	190.2	190.2	191.0	0.8
U	35,800	3,000	47,326	2.2	191.6	191.6	192.6	1.0
V	37,500	960	14,900	7.0	191.6	191.6	192.6	1.0

[1] Feet above confluence with Congaree River
[2] Width within county / total width
[3] Elevation computed without consideration of backwater effects from Congaree River

TABLE 15

FEDERAL EMERGENCY MANAGEMENT AGENCY

LEXINGTON COUNTY, SOUTH CAROLINA

AND INCORPORATED AREAS

FLOODWAY DATA

SALUDA RIVER

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	2,070	290	991	1.7	147.0	147.0	148.0	1.0
B	3,184	330	1,782	0.9	151.6	151.6	152.5	0.9
C	7,917	190	602	2.7	165.3	165.3	165.4	0.1
D	8,415	162	505	3.2	167.7	167.7	168.5	0.8
E	9,422	429	3,099	0.5	176.4	176.4	177.3	0.9
F	10,040	610	3,184	0.5	176.4	176.4	177.3	0.9
G	10,804	340	569	2.9	178.0	178.0	178.7	0.7
H	11,314	350	1,208	1.3	180.9	180.9	181.8	0.9
I	11,666	350	1,233	1.3	181.6	181.6	182.6	1.0
J	12,491	216	807	2.0	183.8	183.8	184.7	0.9
K	13,349	274	1,328	1.2	185.8	185.8	186.5	0.7
L	14,007	87	414	3.9	186.9	186.9	187.7	0.8
M	14,635	175	728	2.2	189.8	189.8	190.8	1.0
N	15,815	130	543	2.6	197.9	197.9	198.8	0.9
O	16,740	109	527	2.6	201.2	201.2	201.8	0.6
P	17,586	136	634	2.2	203.2	203.2	204.2	1.0
Q	18,196	180	807	1.4	204.3	204.3	205.3	1.0
R	19,132	70	311	3.6	206.2	206.2	207.1	0.9
S	19,777	100	225	4.9	212.2	212.2	212.6	0.4
T	20,402	200	699	1.6	216.0	216.0	217.0	1.0
U	21,560	295	2,026	0.3	227.3	227.3	228.3	1.0
V	22,094	100	334	2.1	227.3	227.3	228.2	0.9
W	23,399	100	510	1.8	240.9	240.9	241.8	0.9
X	24,109	50	140	6.6	245.9	245.9	246.8	0.9
Y	24,872	50	142	2.5	254.7	254.7	255.7	1.0
Z	26,002	232	2,011	0.2	278.4	278.4	279.3	0.9

[1] Feet above confluence with Congaree Creek

TABLE 15

FEDERAL EMERGENCY MANAGEMENT AGENCY

LEXINGTON COUNTY, SOUTH CAROLINA

AND INCORPORATED AREAS

FLOODWAY DATA

SAVANA BRANCH

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	2,105	675	2,871	2.1	183.7	183.7	184.6	0.9
B	2,596	647	5,447	1.1	184.4	184.4	185.3	0.9
C	3,494	232	1,749	1.3	184.9	184.9	185.8	0.9
D	4,870	562	3,495	0.6	186.3	186.3	187.2	0.9
E	6,291	170	877	2.5	187.2	187.2	188.0	0.8
F	7,797	110	856	2.6	195.3	195.3	195.3	0.0
G	8,599	81	497	4.4	195.8	195.8	196.3	0.5
H	9,447	300	1,820	1.2	196.6	196.6	197.5	0.9
I	10,133	365	2,157	1.0	196.9	196.9	197.9	1.0
J	10,846	45	245	8.7	196.9	196.9	197.4	0.5
K	11,734	90	704	3.0	201.4	201.4	202.4	1.0
L	12,622	75	213	10.0	204.6	204.6	204.6	0.0
M	13,844	180	963	1.7	209.1	209.1	210.1	1.0
N	14,317	112	486	3.4	210.2	210.2	211.1	0.9
O	16,469	519	3,427	0.5	220.1	220.1	220.1	0.0
P	17,317	237	981	1.8	220.1	220.1	220.1	0.0
Q	18,087	145	267	5.4	221.5	221.5	221.6	0.1
R	18,695	175	677	2.1	224.4	224.4	225.4	1.0
S	19,920	137	475	3.0	229.9	229.9	230.9	1.0
T	20,832	176	690	2.1	232.7	232.7	233.7	1.0
U	21,810	95	424	3.4	235.2	235.2	236.2	1.0
V	23,111	246	1,526	0.9	247.0	247.0	247.0	0.0
W	24,107	39	182	7.9	248.5	248.5	248.6	0.1
X	24,507	89	702	2.1	251.5	251.5	252.5	1.0

[1] Feet above confluence with First Creek

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Y	25,146	269	1,308	1.1	252.4	252.4	253.4	1.0
Z	26,621	387	1,141	0.7	264.6	264.6	264.7	0.1
AA	27,855	50	131	6.0	272.8	272.8	272.9	0.1
AB	28,767	235	1,666	0.5	288.2	288.2	289.2	1.0
AC	29,339	185	833	0.9	288.2	288.2	289.2	1.0
AD	29,780	170	698	1.1	288.3	288.3	289.2	0.9
AE	30,370	120	133	5.9	291.4	291.4	291.4	0.0
AF	31,146	49	134	5.9	301.3	301.3	301.5	0.2

[1] Feet above confluence with First Creek

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	950	225	835	2.8	167.6	166.7[2]	167.7	1.0
B	1,550	170	763	2.9	169.7	169.7	170.7	1.0
C	1,700	165	3,183	0.7	177.4	177.4	178.4	1.0
D	2,350	145	918	2.4	177.7	177.7	178.7	1.0
E	3,050	235	788	2.8	180.7	180.7	181.7	1.0
F	3,500	320	4,512	0.5	192.4	192.4	192.4	0.0
G	4,400	105	182	12.0	196.2	196.2	196.2	0.0
H	4,550	105	741	3.0	200.0	200.0	200.8	0.8
I	5,500	105	379	5.8	204.5	204.5	205.4	0.9
J	5,650	325	1,358	1.6	220.1	220.1	221.1	1.0
K	6,300	190	1,358	1.6	220.1	220.1	221.1	1.0
L	7,100	70	300	5.3	221.3	221.3	222.0	0.7
M	7,200	73	373	3.6	222.3	222.3	223.3	1.0
N	7,400	50	262	5.2	223.4	223.4	224.4	1.0
O	7,650	61	306	4.4	225.6	225.6	226.6	1.0
P	7,875	44	320	4.2	230.7	230.7	231.7	1.0
Q	8,300	70	191	7.1	238.1	238.1	238.3	0.2
R	8,730	68	444	3.1	242.0	242.0	242.9	0.9
S	9,100	50	149	6.7	249.4	249.4	249.7	0.3
T	9,430	62	239	4.2	255.3	255.3	256.2	0.9
U	10,700	39	164	3.2	264.3	264.3	265.2	0.9
V	11,115	60	437	1.1	271.2	271.2	272.1	0.9
W	11,590	27	68	7.3	274.5	274.5	274.5	0.0
X	11,850	24	123	4.0	277.4	277.4	278.3	0.9

[1] Feet above confluence with Saluda River

[2] Elevation computed without consideration of backwater effects from Saluda River

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Y	12,330	18	77	5.4	280.6	280.6	281.6	1.0
Z	12,650	128	768	0.4	286.5	286.5	286.6	0.1
AA	12,900	128	361	0.9	287.0	287.0	287.9	0.9
AB	13,120	95	328	1.0	289.1	289.1	289.4	0.3
AC	13,600	31	65	4.3	291.0	291.0	291.8	0.8
AD	14,095	44	119	2.4	297.9	297.9	298.9	1.0

[1] Feet above confluence with Saluda River

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	3,150	400	1,000	4.8	140.9	127.2[2]	128.2	1.0
B	6,450	314	2,381	2.0	140.9	133.3[3]	134.3	1.0
C	9,200	455	1,983	2.4	140.9	137.7[3]	138.7	1.0
D	12,700	205	910	5.3	144.9	144.9	145.2	0.3
E	13,350	180	1,381	3.5	147.7	147.7	148.3	0.6
F	13,700	260	2,672	1.8	151.0	151.0	151.7	0.7
G	15,600	473	3,637	1.3	151.9	151.9	152.9	1.0
H	17,150	355	2,092	1.9	153.0	153.0	154.0	1.0
I	17,450	440	3,663	1.1	156.4	156.4	157.3	0.9
J	18,850	440	2,705	1.5	157.2	157.2	158.2	1.0
K	19,750	147	1,414	2.8	162.8	162.8	163.6	0.8
L	20,800	443	2,899	2.8	163.3	163.3	164.3	1.0
M	21,100	195	1,618	0.4	168.5	168.5	168.7	0.2
N	21,500	180	1,394	0.4	168.8	168.8	169.6	0.8
O	21,850	450	4,644	0.9	172.5	172.5	173.5	1.0
P	22,300	175	2,362	1.7	172.5	172.5	173.5	1.0
Q	22,500	290	3,270	1.2	177.9	177.9	178.9	1.0
R	23,700	170	910	4.4	178.0	178.0	178.9	0.9
S	24,400	100	404	9.9	182.5	182.5	182.9	0.4
T	24,700	130	1,579	2.5	188.5	188.5	189.5	1.0
U	24,900	350	1,918	2.1	190.8	190.8	191.6	0.8
V	25,300	200	1,193	3.4	191.0	191.0	191.8	0.8
W	25,700	95	713	4.7	193.3	193.3	194.2	0.9
X	25,850	170	2,099	1.6	193.8	193.8	194.8	1.0

[1] Feet above confluence with Congaree Creek

[2] Elevation computed without consideration of flooding controlled by effects from Congaree River

[3] Elevation computed without consideration of backwater effects from Congaree River

TABLE 15

FEDERAL EMERGENCY MANAGEMENT AGENCY

LEXINGTON COUNTY, SOUTH CAROLINA

AND INCORPORATED AREAS

FLOODWAY DATA

SIX MILE CREEK

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Y	26,130	280	1,906	1.7	194.6	194.6	195.5	0.9
Z	27,000	185	1,002	3.3	197.8	197.8	198.8	1.0
AA	27,200	70	476	7.0	200.9	200.9	201.2	0.3
AB	27,500	115	934	3.6	201.9	201.9	202.7	0.8
AC	28,200	125	797	4.2	203.9	203.9	204.9	1.0
AD	29,000	90	501	6.7	207.5	207.5	208.4	0.9
AE	29,400	320	2,930	1.0	214.9	214.9	215.9	1.0
AF	30,850	245	1,724	1.7	215.2	215.2	216.2	1.0
AG	32,500	190	1,110	2.4	222.5	222.5	223.5	1.0
AH	32,850	293	7,637	0.3	244.5	244.5	244.5	0.0
AI	33,350	155	3,472	0.8	244.5	244.5	244.5	0.0
AJ	33,720	180	2,295	1.1	244.6	244.6	244.6	0.0
AK	34,650	200	2,766	0.6	244.6	244.6	244.6	0.0
AL	34,920	226	1,539	1.0	244.6	244.6	244.7	0.1
AM	36,450	113	519	3.1	245.7	245.7	246.7	1.0
AN	38,450	70	180	8.8	260.6	260.6	261.5	0.9

[1] Feet above confluence with Congaree Creek

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	335	40	124	4.6	146.9	146.9	147.6	0.7
B	890	80	574	1.0	149.5	149.5	150.4	0.9
C	2,100	13	51	11.2	158.8	158.8	158.8	0.0
D	2,150	45	284	2.0	161.4	161.4	161.5	0.1
E	2,350	45	242	2.2	161.4	161.4	161.9	0.5
F	2,900	40	163	3.3	164.8	164.8	165.8	1.0
G	3,100	74	230	2.3	169.1	169.1	169.9	0.8

[1] Feet above confluence with Six Mile Creek

TABLE 15

FEDERAL EMERGENCY MANAGEMENT AGENCY

LEXINGTON COUNTY, SOUTH CAROLINA

AND INCORPORATED AREAS

FLOODWAY DATA

SIX MILE CREEK TRIBUTARY 2

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	950	230	2,280	1.0	162.4	162.4	163.4	1.0
B	1,500	115	323	7.2	171.9	171.9	172.6	0.7
C	1,900	105	549	4.2	173.7	173.7	173.9	0.2
D	2,350	100	6,320	0.4	174.3	174.3	175.3	1.0
E	3,600	100	4,120	0.6	174.3	174.3	175.3	1.0
F	5,200	101	543	3.8	175.5	175.5	176.2	0.7
G	5,290	101	732	2.8	177.5	177.5	178.1	0.6
H	5,750	174	820	2.2	178.0	178.0	178.8	0.8
I	5,880	197	1,461	1.2	181.6	181.6	182.6	1.0
J	6,400	137	890	2.0	182.2	182.2	183.2	1.0
K	7,140	77	460	3.9	184.8	184.8	185.3	0.5
L	7,590	86	391	4.5	188.2	188.2	188.9	0.7
M	8,180	38	255	4.3	193.8	193.8	193.8	0.0
N	8,350	67	347	1.3	195.0	195.0	195.0	0.0
O	9,230	200	3,944	0.3	212.1	212.1	212.1	0.0
P	10,030	330	3,830	0.3	212.1	212.1	212.1	0.0
Q	10,600	204	1,406	0.8	212.1	212.1	212.1	0.0
R	11,000	290	2,036	0.5	212.1	212.1	212.1	0.0
S	11,250	291	1,750	0.6	212.1	212.1	212.1	0.0

[1] Feet above confluence with Six Mile Creek

TABLE 15

FEDERAL EMERGENCY MANAGEMENT AGENCY

LEXINGTON COUNTY, SOUTH CAROLINA

AND INCORPORATED AREAS

FLOODWAY DATA

SIX MILE CREEK TRIBUTARY 3

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	50	85	377	4.3	192.7	192.7	193.7	1.0
B	300	120	598	2.7	194.0	194.0	195.0	1.0
C	1,000	95	399	4.0	199.2	199.2	200.0	0.8
D	1,500	140	756	2.1	201.9	201.9	202.9	1.0
E	2,350	60	261	6.2	212.2	212.2	212.5	0.3
F	2,800	150	545	3.0	215.7	215.7	216.5	0.8
G	3,800	93	–[2]	–[2]	224.8	224.8	225.8	1.0

[1] Feet above confluence with Six Mile Creek
[2] Data not available

TABLE 15

FEDERAL EMERGENCY MANAGEMENT AGENCY

LEXINGTON COUNTY, SOUTH CAROLINA

AND INCORPORATED AREAS

FLOODWAY DATA

SIX MILE CREEK TRIBUTARY 5

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	1,490	60	299	7.4	182.4	170.7[3]	171.5	0.8
B	1,670	143	1,221	1.8	182.4	178.4[3]	178.4	0.0
C	2,455	120	673	3.3	182.4	178.8[3]	178.8	0.0
D	2,625	88	586	3.8	182.4	182.0[3]	182.0	0.0
E	3,540	201	907	2.4	183.5	183.5	183.5	0.0
F	4,250	163	518	4.3	187.8	187.8	188.6	0.8
G	4,300	261	1,376	1.6	188.1	188.1	188.9	0.8
H	4,660	192	945	2.3	188.3	188.3	189.1	0.8
I	5,110	234	1,462	1.5	194.0	194.0	194.0	0.0
J	5,680	75	422	8.2	194.6	194.6	194.7	0.1
K	6,120	92	579	6.0	197.8	197.8	198.7	0.9
L	6,340	61	520	6.7	199.2	199.2	199.5	0.3
M	7,130	100	452	7.7	204.0	204.0	204.9	0.9
N	8,000	50	452	6.7	209.1	209.1	209.9	0.8
O	9,600	0 / 124[2]	1,657	2.0	219.7	219.7	219.7	0.0
P	10,450	0 / 180[2]	1,578	2.1	219.8	219.8	220.4	0.6

[1] Feet above confluence with Saluda River

[2] Width within county / total width

[3] Elevation computed without consideration of backwater effects from Saluda River

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	750	58	223	3.2	263.3	263.3	264.3	1.0
B	905	69	261	2.8	264.5	264.5	265.4	0.9
C	1,182	31	115	6.3	267.4	267.4	268.1	0.7
D	1,660	154	365	2.0	270.5	270.5	271.5	1.0
E	1,860	104	212	3.4	271.7	271.7	272.5	0.8
F	1,960	66	193	3.7	272.9	272.9	273.8	0.9
G	2,136	35	151	3.1	275.8	275.8	276.8	1.0

[1] Feet above confluence with Fourteen Mile Creek

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	841	100	335	3.4	336.2	335.1[2]	335.5	0.4
B	1,786	90	332	3.5	340.9	340.9	341.5	0.6
C	3,236	30	164	3.5	351.6	351.6	351.7	0.1

[1] Feet above confluence with Red Bank Creek

[2] Elevation computed without consideration of backwater effects from Red Bank Creek

TABLE 15

FEDERAL EMERGENCY MANAGEMENT AGENCY

LEXINGTON COUNTY, SOUTH CAROLINA

AND INCORPORATED AREAS

FLOODWAY DATA

TRIBUTARY TO RED BANK CREEK

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	2,970	688	4,180	2.3	191.6	179.6[2]	180.5	0.9
B	3,174	793	2,363	4.0	191.6	185.1[2]	185.9	0.8
C	3,303	737	7,096	1.3	191.6	186.0[2]	187.0	1.0
D	4,303	641	5,676	1.7	191.6	186.4[2]	187.4	1.0
E	5,403	698	5,185	1.9	191.6	187.8[2]	188.5	0.7
F	5,803	713	5,471	1.8	191.6	188.3[2]	188.9	0.6
G	7,003	765	4,343	2.2	191.6	190.1[2]	190.5	0.4
H	7,673	790	4,173	2.3	191.9	191.9	192.4	0.5
I	10,553	860	5,559	1.4	195.8	195.8	196.8	1.0
J	11,203	345	1,598	4.8	197.1	197.1	197.4	0.3
K	11,583	405	2,363	3.3	198.9	198.9	199.8	0.9
L	12,163	130	1,144	6.8	201.6	201.6	202.0	0.4
M	12,293	170	1,371	5.6	203.2	203.2	203.4	0.2
N	12,403	300	2,547	3.0	204.2	204.2	204.4	0.2
O	15,558	417	2,511	3.1	210.0	210.0	211.0	1.0
P	15,688	400	2,736	2.8	210.3	210.3	211.3	1.0
Q	19,543	500	4,093	1.8	216.5	216.5	217.5	1.0
R	21,698	440	3,497	2.1	219.1	219.1	220.1	1.0
S	24,118	273	2,036	3.7	222.5	222.5	223.3	0.8
T	24,658	104	811	9.2	223.4	223.4	224.0	0.6
U	24,708	115	835	9.0	225.3	225.3	225.5	0.2
V	25,923	186	1,320	5.7	234.6	234.6	234.6	0.0
W	27,638	370	3,898	1.9	237.0	237.0	237.8	0.8
X	28,408	700	6,296	1.2	237.3	237.3	238.3	1.0

[1] Feet above confluence with Saluda River

[2] Elevation computed without consideration of backwater effects from Saluda River

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
Y	29,923	460	2,821	2.7	238.4	238.4	238.4	1.0
Z	30,263	675	4,633	1.6	239.3	239.3	239.3	0.7
AA	32,243	340	2,526	3.0	241.9	241.9	241.9	0.6
AB	33,043	210	1,402	5.3	244.1	244.1	244.1	0.9
AC	34,263	310	3,306	2.3	250.4	250.4	250.4	0.6
AD	34,843	719	5,552	1.3	250.8	250.8	250.8	0.8
AE	35,163	718	5,513	1.4	251.0	251.0	251.0	0.8
AF	37,523	519	2,808	2.5	255.3	255.3	255.3	0.6
AG	40,198	445	2,880	2.4	262.5	262.5	262.5	0.0
AH	42,123	306	1,852	3.8	268.8	268.8	268.8	0.2
AI	42,823	159	1,408	5.0	272.0	272.0	272.0	0.2
AJ	43,123	330	2,707	2.6	273.0	273.0	273.0	0.4
AK	43,323	585	3,827	1.8	274.6	274.6	274.6	0.2
AL	43,823	585	13,026	0.5	297.4	297.4	297.5	0.1
AM	44,453	548	11,778	0.6	297.4	297.4	297.5	0.1
AN	45,925	412	3,736	1.7	297.4	297.4	297.5	0.1
AO	46,323	440	4,021	1.6	298.9	298.9	299.5	0.6
AP	46,823	135	1,246	5.2	299.3	299.3	299.8	0.5
AQ	47,623	223	1,557	4.2	303.2	303.2	303.5	0.3
AR	49,523	192	1,655	2.0	307.7	307.7	308.1	0.4
AS	49,664	156	1,580	2.1	307.8	307.8	308.2	0.4
AT	50,511	344	3,864	0.9	315.6	315.6	316.3	0.7
AU	51,300	414	4,106	0.8	315.6	315.6	316.3	0.7
AV	51,812	400	3,682	0.9	315.6	315.6	316.3	0.7

[1] Feet above confluence with Saluda River

TABLE 15

FEDERAL EMERGENCY MANAGEMENT AGENCY

LEXINGTON COUNTY, SOUTH CAROLINA

AND INCORPORATED AREAS

FLOODWAY DATA

TWELVE MILE CREEK

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
AW	52,515	99	795	4.2	315.7	315.7	316.0	0.3
AX	53,410	379	3,532	0.9	317.3	317.3	318.3	1.0
AY	53,909	413	3,734	0.9	317.5	317.5	318.5	1.0
AZ	54,886	300	2,126	1.6	318.1	318.1	319.1	1.0
BA	55,360	314	2,196	1.5	318.7	318.7	319.6	0.9
BB	55,848	260	1,871	1.8	319.4	319.4	320.4	1.0
BC	57,647	842	8,889	0.4	329.9	329.9	330.9	1.0
BD	58,675	692	6,347	0.5	329.9	329.9	330.9	1.0
BE	59,333	752	6,151	0.5	329.9	329.9	330.8	0.9
BF	60,294	375	1,839	1.8	329.9	329.9	330.9	1.0
BG	61,343	315	2,176	1.5	330.9	330.9	331.8	0.9
BH	62,112	373	2,327	1.4	331.3	331.3	332.3	1.0
BI	63,009	644	3,894	0.8	331.9	331.9	332.9	1.0
BJ	63,624	384	2,177	1.5	332.3	332.3	333.3	1.0
BK	65,440	75	580	5.6	338.8	338.8	339.5	0.7
BL	66,155	160	1,135	2.8	340.6	340.6	341.6	1.0
BM	66,711	130	766	4.2	341.7	341.7	342.6	0.9
BN	67,496	215	1,344	2.4	344.3	344.3	345.2	0.9
BO	68,241	230	1,208	2.7	345.8	345.8	346.7	0.9
BP	68,932	270	1,745	1.8	347.6	347.6	348.6	1.0
BQ	69,271	230	1,535	2.0	348.4	348.4	349.4	1.0
BR	70,215	415	2,348	1.3	349.5	349.5	350.5	1.0
BS	71,153	230	1,615	1.8	352.5	352.5	353.1	0.6
BT	71,698	300	1,977	1.4	352.7	352.7	353.6	0.9

[1] Feet above confluence with Saluda River

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
BU	72,436	328	1,782	1.6	353.4	353.4	354.4	1.0
BV	72,935	348	1,593	1.8	354.2	354.2	355.2	1.0
BW	74,413	385	1,721	1.7	357.4	357.4	358.4	1.0
BX	74,918	190	913	3.1	358.6	358.6	359.5	0.9
BY	75,908	270	1,441	2.0	361.4	361.4	362.4	1.0
BZ	76,915	190	840	3.4	364.0	364.0	365.0	1.0
CA	78,243	190	991	2.9	369.0	369.0	369.4	0.4
CB	79,028	203	1,043	2.7	370.1	370.1	371.0	0.9
CC	79,781	195	1,209	2.4	372.2	372.2	373.2	1.0
CD	80,743	345	1,913	1.4	374.1	374.1	375.1	1.0
CE	81,476	296	1,722	1.6	375.1	375.1	376.1	1.0
CF	82,372	246	1,094	2.4	376.8	376.8	377.7	0.9
CG	83,576	81	537	3.8	381.2	381.2	382.2	1.0
CH	84,728	158	1,167	1.7	386.1	386.1	387.1	1.0
CI	85,395	153	1,117	1.8	386.8	386.8	387.8	1.0
CJ	86,038	113	745	2.7	387.8	387.8	388.7	0.9
CK	87,031	120	695	2.9	390.6	390.6	391.6	1.0
CL	87,675	125	855	2.4	392.5	392.5	393.4	0.9
CM	88,408	160	893	2.2	394.1	394.1	395.1	1.0
CN	89,285	170	967	2.0	396.1	396.1	397.1	1.0
CO	89,914	95	579	3.4	397.7	397.7	398.7	1.0
CP	91,063	424	4,717	0.4	411.6	411.6	411.6	0.0
CQ	91,754	242	2,040	1.0	411.6	411.6	411.6	0.0
CR	92,404	260	1,516	1.3	411.6	411.6	411.6	0.0

[1] Feet above confluence with Saluda River

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
CS	93,042	180	832	2.4	412.3	412.3	412.5	0.2
CT	93,959	157	929	2.2	414.3	414.3	415.2	0.9
CU	94,415	127	835	2.4	415.3	415.3	416.2	0.9
CV	95,202	95	538	3.8	418.2	418.2	419.1	0.9
CW	95,800	156	865	2.3	421.2	421.2	422.2	1.0
CX	97,100	69	432	3.7	427.0	427.0	428.0	1.0
CY	99,257	240	1,221	1.3	439.0	439.0	439.1	0.1
CZ	100,185	40	183	8.6	439.8	439.8	440.3	0.5

[1] Feet above confluence with Saluda River

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	150	139	731	5.0	210.4	208.5[2]	209.5	1.0
B	630	182	895	4.1	213.0	213.0	214.0	1.0
C	1,422	102	511	7.1	219.7	219.7	220.7	1.0
D	2,065	172	895	4.1	226.7	226.7	227.7	1.0
E	2,506	86	426	8.1	230.3	230.3	231.0	0.7
F	3,043	141	658	5.3	237.0	237.0	238.0	1.0

[1] Feet above confluence with Twelve Mile Creek

[2] Elevation computed without consideration of backwater effects from Twelve Mile Creek

LOCATION		FLOODWAY			1% ANNUAL CHANCE FLOOD WATER SURFACE ELEVATION (FEET NAVD88)			
CROSS SECTION	DISTANCE[1]	WIDTH (FEET)	SECTION AREA (SQ. FEET)	MEAN VELOCITY (FEET/SEC)	REGULATORY	WITHOUT FLOODWAY	WITH FLOODWAY	INCREASE
A	2,375	63	208	4.3	219.8	219.8	220.7	0.9
B	5,460	40	141	6.3	251.3	251.3	251.3	0.0
C	9,410	30	139	5.6	295.7	295.7	295.7	0.0
D	10,480	73	164	4.5	305.6	305.6	305.9	0.3

[1] Feet above confluence with Rawls Creek

Table 16 – Flood Hazard Data for Selected Streams

Flooding Source[1]	Cross Section	Stream Station[2]	1% Annual Chance Flood Discharge (cfs)	1% Annual Chance Water Surface Elevation (feet NAVD88)
Chinquapin Creek	000	0	5,189	335.5
	009	902	5,189	335.5
	015	1,528	5,189	335.6
	021	2,141	5,189	335.7
	026	2,587	5,189	335.9
	032	3,174	5,189	336.1
	034	3,353	5,189	336.1
	039	3,870	5,189	336.9
	042	4,243	5,189	337.8
	049	4,931	5,189	339.7
	052	5,167	5,189	340.0
	056	5,593	5,189	341.2
	060	6,005	5,189	341.9
	065	6,548	5,189	342.8
	070	6,959	5,189	343.4
	075	7,548	5,189	344.6
	081	8,089	5,189	345.6
	086	8,648	5,189	346.1
	091	9,121	5,189	346.1
	095	9,530	5,189	346.2
	101	10,093	5,189	346.3
	105	10,503	5,189	346.4
	112	11,156	5,189	346.5
	118	11,826	5,189	346.8
	123	12,334	5,189	347.4
	129	12,902	5,189	348.3
	134	13,437	5,189	350.7
	140	14,026	5,189	352.0
	148	14,760	5,189	352.4
	151	15,096	5,189	352.6
	156	15,582	5,189	353.1
	164	16,354	5,189	353.5
	172	17,219	5,189	354.0
	184	18,426	5,189	354.8
	188	18,820	5,189	355.6
	196	19,604	5,189	358.4
	199	19,933	5,189	358.6
	205	20,492	5,189	359.1
	210	21,001	5,189	359.3
	216	21,559	5,189	360.4
	220	21,968	5,189	361.3

Table 16 – Flood Hazard Data for Selected Streams – continued

Flooding Source[1]	Cross Section	Stream Station[2]	1% Annual Chance Flood Discharge (cfs)	1% Annual Chance Water Surface Elevation (feet NAVD88)
Chinquapin Creek (continued)	226	22,648	5,189	362.9
	229	22,920	5,189	363.5
	234	23,413	5,189	365.7
	238	23,827	5,189	372.1
	240	23,983	5,189	380.5
	244	24,399	5,189	386.8
	249	24,885	5,189	390.6
	264	26,418	5,189	393.8
	268	26,771	5,189	394.3
	274	27,380	5,189	394.4
	281	28,118	5,189	394.7
	288	28,756	5,189	395.2
	293	29,346	5,189	396.5
	297	29,701	5,189	398.3
	302	30,199	5,189	400.1
	308	30,825	5,189	402.8
	313	31,273	5,189	403.9
	317	31,715	5,189	404.6
	322	32,194	5,189	406.2
	325	32,496	5,189	407.6
	329	32,946	5,189	409.3
	333	33,317	5,189	410.8
	338	33,772	5,189	411.6
	342	34,230	5,189	413.1
	346	34,601	5,189	413.3
	351	35,121	5,189	417.2
	355	35,469	5,189	417.6
	356	35,602	5,189	417.9
	359	35,887	5,189	418.1
	364	36,448	5,189	420.7
	369	36,851	5,189	421.5
	375	37,460	5,189	423.3
	379	37,883	5,189	423.8
	382	38,185	5,189	424.5
	387	38,727	5,189	425.6
	391	39,136	5,189	426.6
	395	39,547	5,189	427.9
	399	39,945	5,189	432.3
	402	40,213	5,189	433.3
	404	40,372	5,189	433.6
	409	40,915	5,189	434.3

Table 16 – Flood Hazard Data for Selected Streams – continued

Flooding Source[1]	Cross Section	Stream Station[2]	1% Annual Chance Flood Discharge (cfs)	1% Annual Chance Water Surface Elevation (feet NAVD88)
Chinquapin Creek (continued)	416	41,588	5,189	435.1
	420	41,952	5,189	435.6
	425	42,527	5,189	436.4
	430	42,998	5,189	437.8
	434	43,358	5,189	438.0
	438	43,794	5,189	441.6
	443	44,282	5,189	443.3
	448	44,814	5,189	444.9
	452	45,160	5,189	446.1
	457	45,715	5,189	450.8
	460	46,042	5,189	454.4
	464	46,437	5,189	456.7
	468	46,815	5,189	459.0
	473	47,313	5,189	463.1
	478	47,759	5,189	465.9
	482	48,248	5,189	470.4
	487	48,658	5,189	472.9
	489	48,850	5,189	484.0
	494	49,425	5,189	484.5
	499	49,947	5,189	484.6
	504	50,386	5,189	489.3
	507	50,731	5,189	495.3
	511	51,123	5,189	495.7
	516	51,597	5,189	499.3
	521	52,080	5,189	503.7
	523	52,339	5,189	507.5
	525	52,466	5,189	516.3
	530	53,010	5,189	519.7
Double Branch	063	6,280	1,169	231.6
	063	6,324	1,169	235.8
	064	6,361	1,169	235.8
	065	6,500	1,169	235.8
	067	6,709	548	235.9
	069	6,877	548	238.9
	069	6,909	548	240.4
	069	6,945	548	240.4
	072	7,151	548	240.9
	075	7,500	548	242.9
	080	8,000	548	245.0
	085	8,500	548	248.9
	087	8,727	415	250.0

Table 16 – Flood Hazard Data for Selected Streams – continued

Flooding Source[1]	Cross Section	Stream Station[2]	1% Annual Chance Flood Discharge (cfs)	1% Annual Chance Water Surface Elevation (feet NAVD88)
Double Branch (continued)	088	8,781	415	254.1
	088	8,839	415	254.1
	090	9,000	415	254.2
	094	9,399	415	255.3
	098	9,791	415	258.7
	099	9,863	415	260.8
	099	9,939	415	260.8
	103	10,295	415	261.1
Double Branch Tributary 1	000	41	261	249.4[3]
	002	224	261	250.9
	005	487	261	253.1
	008	843	261	256.0
	012	1,187	261	258.3
	014	1,444	261	260.5
	015	1,500	261	264.2
	015	1,540	261	264.2
	017	1,673	261	264.3
	019	1,916	261	266.3
Dry Creek	085	8,500	2,146	135.5
	090	9,000	2,146	137.1
	095	9,500	2,146	139.1
	100	10,000	2,146	140.8
	106	10,606	2,146	143.0
	107	10,696	2,146	147.1
	108	10,750	2,146	147.1
	108	10,771	2,146	147.1
	109	10,937	2,146	149.7
	111	11,059	2,146	149.7
	115	11,500	1,296	149.7
	120	12,000	1,276	149.8
	125	12,500	1,276	149.9
	130	13,000	1,276	150.1
	135	13,500	1,276	151.9
	140	14,000	1,276	153.9
	145	14,500	1,276	155.5
	150	15,000	1,276	157.1
	155	15,500	1,276	159.0
	159	15,933	1,276	160.6
	160	16,027	1,276	162.7
	161	16,125	1,276	162.7
	165	16,470	1,276	163.1

Table 16 – Flood Hazard Data for Selected Streams – continued

Flooding Source[1]	Cross Section	Stream Station[2]	1% Annual Chance Flood Discharge (cfs)	1% Annual Chance Water Surface Elevation (feet NAVD88)
Dry Creek (continued)	165	16,548	1,276	169.5
	167	16,679	1,276	169.5
	172	17,220	1,276	169.5
	176	17,648	1,276	171.3
	180	18,000	1,276	174.8
	185	18,451	1,276	177.9
	187	18,728	768	180.9
	190	19,000	768	183.2
	195	19,500	768	187.8
	200	20,000	768	192.5
	202	20,239	768	195.9
	203	20,269	768	197.2
	203	20,303	768	197.2
	205	20,500	768	198.4
	210	21,000	768	203.8
	215	21,500	768	210.1
	219	21,889	768	218.0
	222	22,165	768	222.7
Dry Creek Tributary 3	005	532	34	178.1
	008	782	34	180.6
	010	957	34	182.6
	011	1,107	34	185.1
	013	1,333	34	186.8
	015	1,507	34	193.1
	016	1,619	34	194.3
	018	1,814	34	195.8
	021	2,050	34	197.7
	023	2,297	34	199.8
	025	2,527	34	203.7
Kinley Creek	190	19,000	1,664	251.7
	195	19,500	1,664	251.7
	200	20,000	1,664	251.7
	205	20,500	1,664	251.7
	210	21,000	1,664	251.7
	215	21,500	1,664	251.7
	220	22,000	1,131	252.6
	225	22,500	1,131	254.1
	230	23,000	1,131	258.0
	235	23,500	1,131	261.4
Long Branch	028	2,807	772	399.2
	030	3,033	772	399.6
	031	3,149	772	400.6

Table 16 – Flood Hazard Data for Selected Streams – continued

Flooding Source[1]	Cross Section	Stream Station[2]	1% Annual Chance Flood Discharge (cfs)	1% Annual Chance Water Surface Elevation (feet NAVD88)
Long Branch (continued)	032	3,186	772	403.5
	032	3,203	772	403.5
	035	3,500	772	405.5
	040	4,000	772	408.9
	045	4,500	772	413.4
	047	4,674	486	416.2
	049	4,917	486	418.4
	053	5,283	486	420.9
	056	5,600	486	426.5
	060	6,000	486	430.4
	062	6,191	486	435.0
	064	6,418	486	436.8
	068	6,822	486	442.4
	073	7,253	486	449.0
	077	7,699	486	453.4
	078	7,794	486	465.9
	079	7,890	486	465.9
	084	8,360	486	465.9
	086	8,552	486	465.9
	087	8,663	486	467.3
North Fork Edisto River	000	9	18,717	234.7
	004	415	18,717	235.4
	007	678	18,717	235.9
	010	1,028	18,717	236.3
	018	1,762	18,717	237.3
	021	2,149	18,717	237.9
	026	2,626	18,717	238.5
	032	3,200	18,717	239.0
	037	3,669	18,717	239.3
	044	4,438	18,717	239.7
	050	4,966	18,717	240.0
	061	6,051	18,717	240.6
	073	7,281	18,717	241.3
	099	9,897	17,992	242.4
	112	11,152	17,992	242.9
	123	12,259	17,992	243.3
	131	13,063	17,992	243.7
	138	13,787	17,992	244.1
	156	15,587	17,992	245.0
	176	17,567	17,992	245.9
	186	18,624	17,992	246.3

Table 16 – Flood Hazard Data for Selected Streams – continued

Flooding Source[1]	Cross Section	Stream Station[2]	1% Annual Chance Flood Discharge (cfs)	1% Annual Chance Water Surface Elevation (feet NAVD88)
North Fork Edisto River (continued)	195	19,514	17,992	246.5
	204	20,356	17,992	246.8
	208	20,810	17,992	246.9
	214	21,412	17,992	247.0
	223	22,331	17,992	247.3
	235	23,532	17,992	247.8
	247	24,732	17,992	248.4
	259	25,941	17,992	249.0
	267	26,746	17,992	249.5
	275	27,537	17,992	250.0
	290	29,033	17,992	251.1
	300	30,014	17,992	251.8
	305	30,491	17,992	252.1
	316	31,618	17,992	252.6
	325	32,467	17,992	253.0
	333	33,318	17,992	253.5
	343	34,302	17,992	254.0
	353	35,302	17,992	254.4
	361	36,067	17,992	254.7
	372	37,151	17,992	255.1
	386	38,631	17,992	255.6
	396	39,552	17,992	255.8
	408	40,848	17,992	256.2
	418	41,798	17,992	256.5
	426	42,611	17,992	256.7
	431	43,147	17,992	256.9
	438	43,822	17,992	257.0
	453	45,345	15,681	257.4
	471	47,073	15,681	257.7
	477	47,714	15,681	257.9
	486	48,627	15,681	258.1
	497	49,739	15,681	258.6
	503	50,297	15,681	258.8
	510	51,000	15,681	259.0
	516	51,574	15,681	259.2
	521	52,124	15,681	259.2
	529	52,897	15,681	262.0
	538	53,767	15,681	262.1
	539	53,856	15,681	262.6
	548	54,832	15,681	262.7
	562	56,237	15,681	262.9

Table 16 – Flood Hazard Data for Selected Streams – continued

Flooding Source[1]	Cross Section	Stream Station[2]	1% Annual Chance Flood Discharge (cfs)	1% Annual Chance Water Surface Elevation (feet NAVD88)
North Fork Edisto River (continued)	570	57,025	15,681	263.0
	580	57,951	15,681	263.1
	587	58,670	15,681	263.1
	598	59,815	15,681	263.3
	608	60,815	15,681	263.4
	612	61,243	15,681	263.5
	625	62,481	12,625	263.6
	639	63,920	12,625	263.7
	644	64,433	12,625	263.8
	651	65,120	12,625	264.0
	662	66,175	12,625	264.3
	669	66,884	12,625	264.4
	673	67,315	12,625	264.5
	678	67,838	12,625	264.7
	688	68,813	12,625	264.9
	702	70,201	12,625	265.4
	709	70,875	12,625	265.6
	720	72,042	12,625	266.2
	727	72,730	12,625	266.5
	733	73,319	12,625	266.8
	744	74,384	12,625	267.4
	753	75,335	12,625	267.8
	760	75,971	12,625	268.2
	768	76,753	12,625	268.5
	777	77,738	12,625	268.9
	789	78,886	12,625	269.3
	797	79,683	12,625	269.6
	803	80,251	12,625	269.8
	807	80,736	12,625	270.3
	811	81,149	12,625	270.4
	818	81,780	12,625	270.7
	825	82,490	12,625	271.0
	839	83,909	12,625	271.7
	853	85,263	12,625	272.4
	858	85,764	12,625	272.6
	862	86,219	12,625	272.9
	869	86,942	12,625	273.3
	881	88,065	12,625	273.7
	888	88,802	12,625	274.0
	893	89,323	12,625	274.3
	900	89,982	12,625	274.7

Table 16 – Flood Hazard Data for Selected Streams – continued

Flooding Source[1]	Cross Section	Stream Station[2]	1% Annual Chance Flood Discharge (cfs)	1% Annual Chance Water Surface Elevation (feet NAVD88)
North Fork Edisto River (continued)	905	90,452	12,625	274.9
	909	90,889	12,625	275.1
	914	91,405	12,625	275.4
	922	92,151	12,625	275.8
	930	92,958	12,625	276.2
	937	93,659	12,625	276.6
	947	94,671	12,625	277.2
	958	95,845	12,625	277.7
	970	96,956	12,625	278.3
	979	97,864	12,625	278.8
	985	98,499	12,625	279.2
	996	99,590	12,625	279.6
	1003	100,304	12,625	279.9
	1011	101,127	12,625	280.2
	1018	101,783	12,625	280.9
	1026	102,587	12,625	281.2
	1037	103,667	12,625	281.5
	1045	104,472	12,625	281.8
	1056	105,623	12,625	282.3
	1068	106,766	12,625	282.9
	1077	107,697	12,625	283.6
	1083	108,272	12,625	284.1
	1092	109,159	12,625	284.8
	1101	110,125	12,625	285.3
	1111	111,113	12,625	285.7
	1121	112,064	12,625	286.0
	1125	112,537	12,625	286.3
	1132	113,220	12,625	286.6
	1142	114,209	12,625	287.2
	1149	114,890	12,625	287.6
	1156	115,569	12,625	288.1
	1164	116,350	12,625	288.5
	1173	117,304	12,625	289.0
	1178	117,782	12,625	289.8
	1185	118,517	12,625	290.1
	1192	119,212	12,625	290.4
	1201	120,060	12,625	290.7
	1211	121,148	12,625	291.2
	1225	122,492	12,625	291.9
	1236	123,579	12,625	292.6
	1245	124,480	12,625	293.2

Table 16 – Flood Hazard Data for Selected Streams – continued

Flooding Source[1]	Cross Section	Stream Station[2]	1% Annual Chance Flood Discharge (cfs)	1% Annual Chance Water Surface Elevation (feet NAVD88)
North Fork Edisto River (continued)	1258	125,809	12,625	293.9
	1267	126,664	12,625	294.4
	1274	127,421	12,625	294.8
	1288	128,752	12,625	295.4
	1298	129,790	10,093	295.7
	1304	130,387	10,093	295.9
	1312	131,243	10,093	296.3
	1321	132,058	10,093	296.7
	1329	132,880	10,093	297.2
	1334	133,420	10,093	297.5
	1339	133,920	10,093	297.8
	1344	134,420	10,093	298.0
	1348	134,841	10,093	298.3
	1354	135,365	10,093	298.6
	1363	136,318	10,093	299.4
	1370	136,960	10,093	300.0
	1377	137,728	10,093	300.6
	1382	138,178	10,093	301.0
	1389	138,920	10,093	301.8
	1394	139,420	10,093	302.5
	1400	140,049	10,093	303.6
	1405	140,461	10,093	303.7
	1411	141,112	10,093	304.3
	1419	141,920	10,093	305.0
	1425	142,521	10,093	305.4
	1433	143,316	10,093	306.0
	1441	144,077	10,093	306.6
	1447	144,714	10,093	307.0
	1453	145,259	10,093	307.5
	1459	145,885	10,093	308.0
	1463	146,345	10,093	308.3
	1474	147,419	10,093	308.8
	1482	148,222	10,093	309.3
	1489	148,943	10,093	309.7
	1499	149,866	10,093	310.1
	1504	150,420	10,093	310.5
	1509	150,920	10,093	310.9
	1515	151,493	10,093	311.3
	1521	152,099	10,093	312.0
	1528	152,836	10,093	312.9
	1534	153,377	10,093	313.5

Table 16 – Flood Hazard Data for Selected Streams – continued

Flooding Source[1]	Cross Section	Stream Station[2]	1% Annual Chance Flood Discharge (cfs)	1% Annual Chance Water Surface Elevation (feet NAVD88)
North Fork Edisto River (continued)	1539	153,920	10,093	314.2
	1544	154,448	10,093	314.7
	1549	154,940	10,093	315.1
	1553	155,342	10,093	316.1
	1559	155,941	10,093	316.6
	1564	156,449	10,093	316.9
	1569	156,871	10,093	317.1
	1576	157,649	10,093	317.5
	1584	158,350	10,093	318.0
	1589	158,921	10,093	318.6
	1597	159,657	10,093	319.4
	1602	160,153	10,093	319.9
	1606	160,647	10,093	320.2
	1613	161,326	10,093	320.8
	1620	162,030	10,093	321.2
	1627	162,671	10,093	321.7
	1635	163,458	10,093	322.5
	1640	163,983	10,093	323.1
	1645	164,518	10,093	324.0
	1651	165,063	10,093	325.1
	1655	165,459	10,093	328.1
	1659	165,888	10,093	333.9
	1664	166,409	10,093	335.2
	1672	167,229	10,093	335.3
	1678	167,826	10,093	335.4
	1681	168,139	5,221	335.5
Risters Creek	096	9,646	2,533	296.0
	101	10,106	2,517	297.7
	106	10,566	2,517	299.0
	111	11,073	2,517	300.9
	115	11,500	2,517	301.9
	121	12,130	2,517	304.1
	124	12,388	2,425	304.7
	128	12,812	2,425	306.5
	131	13,147	2,296	307.1
	136	13,605	2,296	308.7
	141	14,112	2,296	310.2
	145	14,500	2,296	311.3
	146	14,564	2,296	313.1
	146	14,608	2,296	313.1
	148	14,820	2,296	313.7

Table 16 – Flood Hazard Data for Selected Streams – continued

Flooding Source[1]	Cross Section	Stream Station[2]	1% Annual Chance Flood Discharge (cfs)	1% Annual Chance Water Surface Elevation (feet NAVD88)
Risters Creek Tributary	003	282	814	313.3[3]
	008	788	814	316.4
	011	1,059	796	319.0
	011	1,112	796	324.4
	011	1,147	796	324.4
	014	1,383	796	324.5
	019	1,883	796	327.7
	024	2,383	796	332.8
Rocky Creek	169	16,939	1,412	362.3
	172	17,166	1,412	362.4
	175	17,500	1,412	362.5
	180	18,000	1,412	363.0
	183	18,257	1,412	363.9
	186	18,551	466	364.6
	187	18,659	466	365.0
	187	18,745	466	365.0
	190	19,000	466	368.0
	194	19,431	466	370.6
	195	19,550	466	370.7
	196	19,571	466	372.1
	196	19,601	466	372.1
	199	19,874	466	372.7
	202	20,211	466	375.2
	205	20,500	466	376.2
	210	21,000	466	379.4
	215	21,500	466	385.0
	219	21,887	466	387.3
	219	21,949	466	390.8
	220	21,980	466	390.8
	221	22,051	334	395.0
	221	22,148	334	395.0
	224	22,388	334	395.0
	227	22,674	334	395.0
	230	23,000	334	397.5
	235	23,500	334	404.0
	240	24,000	334	408.7
	245	24,500	334	414.0
	249	24,857	334	420.0
Saluda River Tributary 1	005	500	1,165	200.2[4]
	010	1,000	1,108	200.2[4]
	015	1,500	1,108	200.2[4]

Table 16 – Flood Hazard Data for Selected Streams – continued

Flooding Source[1]	Cross Section	Stream Station[2]	1% Annual Chance Flood Discharge (cfs)	1% Annual Chance Water Surface Elevation (feet NAVD88)
Saluda River Tributary 1 (continued)	020	2,000	1,108	200.2[3]
	025	2,500	1,108	200.2[3]
	030	3,000	1,108	200.2[3]
	035	3,500	1,108	200.2[3]
	038	3,771	1,108	200.2[3]
	042	4,151	704	200.2[3]
	045	4,500	704	200.2[3]
	050	5,000	704	202.4
	055	5,500	704	207.0
	059	5,869	704	209.5
	059	5,920	704	216.9
	060	5,970	704	216.9
	061	6,120	676	216.9
	065	6,500	676	217.2
	070	7,000	676	223.5
	075	7,500	676	227.7
	078	7,758	676	230.9
	079	7,918	676	235.8
	080	7,971	676	244.3
	080	8,020	676	244.3
	081	8,106	676	244.3
Saluda River Tributary 1-1	005	500	251	200.2[4]
	010	1,000	251	200.2[3]
	015	1,500	251	201.3
	020	2,000	251	212.3
	024	2,437	251	227.3
	025	2,493	251	236.9
	025	2,547	251	236.9
	029	2,913	251	241.3
Saluda River Tributary 1-2	002	192	651	200.2[3]
	005	500	651	200.2[3]
	010	1,000	651	203.7
	015	1,500	651	207.0
	018	1,794	651	209.5
	021	2,122	532	214.6
	025	2,500	532	217.0
	028	2,790	532	219.5
	028	2,820	532	221.9
	028	2,841	532	221.9

Table 16 – Flood Hazard Data for Selected Streams – continued

Flooding Source[1]	Cross Section	Stream Station[2]	1% Annual Chance Flood Discharge (cfs)	1% Annual Chance Water Surface Elevation (feet NAVD88)
Saluda River Tributary 1-2 (continued)	031	3,129	532	223.6
	034	3,383	532	225.0
	034	3,445	532	235.4
	035	3,513	532	235.4
	039	3,864	532	235.4
	043	4,296	532	241.5
Saluda River Tributary 1-2-2	001	69	244	213.5[3]
	002	227	244	216.7
	004	379	244	219.4
	004	403	244	222.2
	004	423	244	222.2
	005	513	244	222.3
	006	557	244	226.2
	006	625	244	226.2
	009	881	244	233.3
Toms Branch	079	7,949	540	130.3[3]
	084	8,399	540	130.3[3]
	087	8,719	540	130.3[3]
	091	9,070	540	130.6
	097	9,725	540	132.4
	102	10,181	540	133.5
	105	10,500	540	134.2
	106	10,575	540	136.2
	106	10,620	540	136.2
	110	11,000	540	136.8
	114	11,395	540	137.2
	117	11,728	540	137.6
	122	12,161	540	138.3
	124	12,378	540	139.2
	125	12,500	540	139.2
	130	13,000	540	141.0
	135	13,500	540	142.5
	140	14,025	540	144.3
	143	14,295	362	145.0
	147	14,687	362	145.7
	150	15,000	362	146.6
	155	15,500	362	149.3
	161	16,129	362	151.3
	162	16,197	362	151.5
	162	16,224	362	151.5

Table 16 – Flood Hazard Data for Selected Streams – continued

Flooding Source[1]	Cross Section	Stream Station[2]	1% Annual Chance Flood Discharge (cfs)	1% Annual Chance Water Surface Elevation (feet NAVD88)
Toms Branch (continued)	163	16,284	355	158.9
	163	16,349	355	158.9
	165	16,500	355	158.9
	170	17,000	355	158.9
	175	17,500	355	158.9
	180	18,000	355	158.9
	185	18,500	355	159.0
	190	19,000	355	160.3
	193	19,303	355	161.9
	194	19,377	291	170.5
	195	19,500	291	170.5
	198	19,786	291	170.5
	203	20,348	291	170.6
	208	20,785	291	170.6
	209	20,899	291	183.6
	210	20,966	291	183.6
	215	21,500	291	183.6
	220	22,000	291	183.6
	225	22,473	291	183.6
	225	22,526	291	183.6
	226	22,567	291	183.6
	229	22,870	291	183.6
	232	23,234	160	183.6
	233	23,285	160	188.4
	234	23,384	160	188.4
	237	23,710	160	188.4
	240	24,000	160	188.4
	244	24,412	160	188.4
	247	24,674	160	189.5
	251	25,057	160	194.1
	255	25,541	160	198.1
	259	25,924	160	200.1
	260	25,963	160	203.7
	260	26,000	160	203.7
	265	26,500	160	205.1
	270	27,000	160	207.9
	275	27,500	160	209.7
	280	28,000	160	217.0
	285	28,500	160	222.7
	287	28,697	160	226.2
	287	28,739	160	232.7
	288	28,785	160	232.7

Table 16 – Flood Hazard Data for Selected Streams – continued

Flooding Source[1]	Cross Section	Stream Station[2]	1% Annual Chance Flood Discharge (cfs)	1% Annual Chance Water Surface Elevation (feet NAVD88)
Toms Branch (continued)	290	29,000	160	232.7
	294	29,428	160	235.8
Toms Branch Tributary 1	007	698	352	144.6
	011	1,134	352	147.6
	015	1,500	352	149.1
	020	2,000	352	151.7
	026	2,567	352	155.9
	027	2,660	352	158.9
	027	2,721	352	158.9
	029	2,875	352	159.2
	031	3,080	352	160.9
	031	3,119	352	160.9
	032	3,159	352	160.9
	033	3,319	352	161.0
	036	3,615	352	161.2
	042	4,213	352	161.8
	047	4,727	352	162.3
Turkey Creek	001	106	1,018	305.4[3]
	004	360	1,018	305.4[3]
	004	407	1,018	305.4[3]
	004	443	1,018	305.4[3]
	009	899	1,018	307.9
	013	1,270	1,018	311.5
	013	1,296	1,018	313.1
	013	1,317	1,018	313.1
	016	1,619	1,018	314.8
	019	1,915	1,018	315.2
	022	2,219	1,018	315.5
	025	2,502	1,018	317.0
	026	2,561	1,018	323.2
	026	2,598	1,018	323.2
	028	2,765	1,018	323.2
	028	2,824	1,018	323.2
	030	3,000	1,018	323.2
	035	3,500	1,018	323.3
	040	4,000	1,018	323.3
	045	4,500	1,018	323.5
	050	5,000	1,018	323.8
	054	5,406	500	325.4
	055	5,521	500	330.5
	056	5,589	500	330.5

Table 16 – Flood Hazard Data for Selected Streams – continued

Flooding Source[1]	Cross Section	Stream Station[2]	1% Annual Chance Flood Discharge (cfs)	1% Annual Chance Water Surface Elevation (feet NAVD88)
Turkey Creek (continued)	060	6,000	500	330.5
	065	6,500	500	330.5
	068	6,828	500	331.4
	072	7,157	500	335.9
	072	7,227	500	340.8
	073	7,299	500	340.8
	075	7,500	500	340.9
	080	8,000	500	341.0
	084	8,354	500	341.1
	089	8,876	500	341.4
Wateree Creek	313	31,345	1,981	290.9
	321	32,062	1,981	292.7
	325	32,531	1,807	293.8
Wateree Creek Tributary 7	028	2,832	1,388	284.2
	032	3,158	1,388	288.6
	034	3,404	1,388	290.3

[1] This table reflects all modeled cross-sections; some cross-sections shown in this table may not appear on the map

[2] Feet above mouth

[3] Elevation includes backwater effects

[4] Elevation includes flooding controlled by effects

5.0 INSURANCE APPLICATIONS

For flood insurance rating purposes, flood insurance zone designations are assigned to a community based on the results of the engineering analyses. The zones are as follows:

Zone A

Zone A is the flood insurance rate zone that corresponds to the 1-percent-annual-chance floodplains that are determined in the FIS by approximate methods. Because detailed hydraulic analyses are not performed for such areas, no BFEs or depths are shown within this zone.

Zone AE

Zone AE is the flood insurance rate zone that corresponds to the 1-percent-annual-chance floodplains that are determined in the FIS by detailed methods. In most instances, whole-foot BFEs derived from the detailed hydraulic analyses are shown at selected intervals within this zone.

Zone X

Zone X is the flood insurance rate zone that corresponds to areas outside the 0.2-percent-annual-chance floodplain, areas within the 0.2-percent-annual-chance floodplain, areas of 1-percent-annual-chance flooding where average depths are less than 1.0 foot, areas of 1-percent-annual-chance flooding where the contributing drainage area is less than 1.0 square mile, and areas protected from the 1-percent-annual-chance flood by levees. No BFEs or depths are shown within this zone.

6.0 FLOOD INSURANCE RATE MAP

The FIRM is designed for flood insurance and floodplain management applications.

For flood insurance applications, the map designates flood insurance rate zones as described in Section 5.0 and, in the 1-percent-annual-chance floodplains that were studied by detailed methods, shows selected whole-foot BFEs or average depths. Insurance agents use the zones and BFEs in conjunction with information on structures and their contents to assign premium rates for flood insurance policies.

For floodplain management applications, the map shows by tints, screens, and symbols, the 1- and 0.2-percent-annual-chance floodplains, floodways and the locations of selected cross-sections used in the hydraulic analyses and floodway computations.

The countywide FIRM presents flooding information for the entire geographic area of Lexington County. Previously, FIRM panels were prepared for each identified flood-prone incorporated community and the unincorporated areas of the county. This countywide FIRM also includes flood hazard information that was presented separately on Flood Boundary and Floodway Maps (FBFMs), where applicable. Historical data relating to the maps prepared for each community are presented in Table 17, "Community Map History".

7.0 OTHER STUDIES

This is a multi-volume FIS. Each volume may be revised separately, in which case it supersedes the previously printed volume. Users should refer to the Table of Contents in Volume 1 for the current effective date of each volume; volumes bearing these dates contain the most up-to-date flood hazard data.

FIS reports have been prepared for Richland County, South Carolina, and Incorporated Areas, Aiken County, South Carolina, and Incorporated Areas, Calhoun County, South Carolina, and Incorporated Areas, Newberry County, South Carolina, and Incorporated Areas, Orangeburg County, South Carolina, and Incorporated Areas, and Saluda County, South Carolina, and Incorporated Areas (Reference 1, 39, 40, 41, 42, & 43).

Because it is based on more up-to-date analyses, this FIS supersedes the previously printed countywide FIS for Lexington County, South Carolina, and Incorporated Areas (Reference 44).

COMMUNITY NAME	INITIAL IDENTIFICATION	FLOOD HAZARD BOUNDARY MAP REVISIONS DATE	FIRM EFFECTIVE DATE	FIRM REVISIONS DATE
Batesburg Leesville, Town of	June 28, 1974	—	June 10, 1977	July 17, 1995 February 9, 2000
Cayce, City of	May 1, 1974	April 30, 1976	May 1, 1980	January 5, 1989 July 17, 1995 February 9, 2000 February 20, 2002
Chapin, Town of[1]	September 22, 1978	—	July 17, 1995	February 9, 2000
Columbia, City of	June 28, 1974	October 22, 1976	September 2, 1981	September 1, 1983 February 4, 1987 January 19, 1994 July 17, 1995 February 9, 2000
Gaston, Town of[1]	July 17, 1995	—	July 17, 1995	February 9, 2000
Gilbert, Town of	October 25, 1974	—	July 17, 1995	February 9, 2000
Irmo, Town of	May 17, 1974	April 30, 1976 January 13, 1978	May 1, 1980	January 3, 1985 April 16, 1991 July 17, 1995 February 9, 2000
Lexington, Town of	June 7, 1974	December 10, 1976 July 22, 1977	May 1, 1980	July 17, 1995 February 9, 2000

[1] No Special Flood Hazard Areas Identified

TABLE 17

FEDERAL EMERGENCY MANAGEMENT AGENCY

LEXINGTON COUNTY, SC

AND INCORPORATED AREAS

COMMUNITY MAP HISTORY

COMMUNITY NAME	INITIAL IDENTIFICATION	FLOOD HAZARD BOUNDARY MAP REVISIONS DATE	FIRM EFFECTIVE DATE	FIRM REVISIONS DATE
Lexington County (Unincorporated Areas)	September 6, 1974	June 30, 1978	June 15, 1981	December 2, 1988 July 17, 1995 February 9, 2000
Pelion, Town of	August 9, 1974	June 4, 1976	July 17, 1995	February 9, 2000
Pine Ridge, Town of	June 21, 1974	August 6, 1976	March 18, 1980	December 2, 1988 July 17, 1995 February 9, 2000 February 20, 2002
South Congaree, Town of	May 17, 1974	June 11, 1976	September 28, 1979	December 2, 1988 July 17, 1995 February 9, 2000
Springdale, Town of	June 28, 1974	July 30, 1976 June 3, 1977	May 1, 1980	July 17, 1995 February 9, 2000
Summit, Town of[1]	July 17, 1995	—	July 17, 1995	February 9, 2000
Swansea, Town of	June 7, 1974	—	June 10, 1977	July 17, 1995 February 9, 2000
West Columbia, City of	June 28, 1974	July 9, 1976 June 3, 1977	February 15, 1979	October 8, 1982 July 17, 1995 February 9, 2000 February 20, 2002

[1] No Special Flood Hazard Areas Identified

TABLE 17

FEDERAL EMERGENCY MANAGEMENT AGENCY

LEXINGTON COUNTY, SC

AND INCORPORATED AREAS

COMMUNITY MAP HISTORY

8.0 LOCATION OF DATA

Information concerning the pertinent data used in the preparation of this study can be obtained by contacting FEMA, Mitigation Division, Koger Center - Rutgers Building, 3003 Chamblee Tucker Road, Atlanta, Georgia 30341.

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